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### ANALYSIS OF DAMPED PLATES

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# AEROSPACE STRUCTURES INFORMATION AND ANALYSIS CENTER

OPERATED FOR THE AIRFORCE FLIGHT DYNAMICS LABORATORY BY ANAMET LABORATORIES, INC.

This report presents three design-oriented methods for the dynamic analysis of sandwich plates, i.e., laminated plates incorporating a core layer of viscoelastic material for vibration damping. The methods are complementary in that each represents a different trade-off between generality, accuracy, and cost of use.

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Prepared by:

David A. Kienholz, Ph.D.

Senior Engineer

Conor D. Johnson, Ph.D.

Principal Engineer

Jatin Parekh Engineer

Approved by:

E. B. Flora Program Manager

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#### 1.0 INTRODUCTION

The use of viscoelastic materials for vibration control has gained wide acceptance, particularly in the aerospace industry. The advantages of the method are many and well documented. However, the effectiveness of any damping treatment is critically dependent on its geometry and on the properties of the viscoelastic material. An uninformed choice can add cost and weight but fail to solve the vibration problem. Some recent advances in analysis techniques utilize finite element methods to provide a more reliable and systematic approach to the design of damping treatments [1,2,3].

Flat plate sections are structural elements which are often good candidates for integral or add-on damping treatments. Such treatments can be highly effective in solving problems of fatigue, acoustic noise radiation, or other undesirable effects of resonant vibration. One of the most weight-efficient forms of damping for plate applications is a thin layer of a viscoelastic material constrained between two metal face sheets to form a sandwich. The purpose of this report is to describe several methods for the analysis and design of sandwich plate structural elements.

Three methods are presented. Each represents a different trade-off of cost of use, accuracy, and generality.

The first and most general technique is called the Modal Strain Energy (MSE) method. It is implemented in MSC/NASTRAN and is applicable to a wide range of structural forms in addition to plates. Basically, it involves modeling the viscoelastic material in a damped structure with solid elements and the metallic material with solid, plate, shell, or other elements as appropriate. All materials are treated initially as being purely elastic (i.e., incapable of energy dissipation). Normal mode properties are calculated and the strain energy distribution associated with each mode shape is used to calculate an approximate value for the modal loss factor. The theoretical basis of the MSE method is

described later in this report, along with practical considerations for modeling of sandwich plates using MSC/NASTRAN. The basic assumptions of the method are verified by comparisons with an exact closed form solution for the case of a simply supported, unriveted, sandwich rectangular plate.

The second method is essentially a set of design charts for sandwich plates. They allow a designer to obtain the modal frequencies and loss factors of a wide variety of sandwich plates with only simple hand calculations. The charts were compiled from a large number of NASTRAN runs using the modal strain energy method. They are plotted in dimensionless form for generality and give modal properties for the first four modes of a rectangular plate with various boundary conditions. The boundary conditions and the ranges of the dimensionless variables are chosen to be typical of the situations that a designer might commonly encounter in practice.

The third method is a simple, inexpensive technique for designing a treatment to damp the higher order flexural modes of a plate. In this case it may be shown that the boundary conditions are relatively unimportant. A closed form solution is used to give the modal loss factor as a function of natural frequency. The method is implemented in an interactive FORTRAN program and comparisons are made with NASTRAN modal strain energy results to illustrate the effect of the approximations.

#### 2.0 MODAL STRAIN ENERGY METHOD

#### 2.1 OVERVIEW

The discretized equations of motion for a damped structure are usually written in the form

$$\underbrace{\mathbf{M}\mathbf{x}}_{\mathbf{x}} + \mathbf{C}\mathbf{\dot{x}}_{\mathbf{x}} + \mathbf{K}\mathbf{x}_{\mathbf{x}} = \mathbf{\dot{x}}(\mathbf{t}) \tag{1}$$

where

 $\widetilde{M}, \widetilde{C}, \widetilde{K} = \text{physical coordinate mass, damping, and stiffness matrices (all real and constant)}$ 

 $x, \dot{x}, \ddot{x} =$  vectors of nodal displacements, velocities, and accelerations

1 = vector of applied node loads

The essence of the modal strain energy method is that it does not attempt to find the damping matrix C. This would be impractical for most real structures and, furthermore, would produce a system of equations which would be very costly to solve. Rather, in the MSE method, one assumes that the damped structure can be represented in terms of the real normal modes of the associated undamped system if appropriate damping terms are inserted into the uncoupled modal equations of motion. That is:

$$\ddot{\alpha}_{r} + \eta^{(r)} \omega_{r} \dot{\alpha}_{r} + \omega_{r}^{2} \alpha_{r} = \ell_{r}(t)$$
(2)

$$\chi = \sum_{r} \phi^{(r)} \alpha_r(t) \qquad r = 1,2,3$$
 (3)

where

 $\alpha_r = r'$ th modal coordinate

 $\omega_{\mathbf{r}}$  = natural radian frequency of the r'th mode

 $_{n}(r) = loss factor of the r'th mode$ 

Equations (2) and (3) imply that the physical coordinate damping matrix C of Eq. (1) need not be explicitly calculated but that it can be diagonalized, at least approximately, by the same real modal matrix that diagonalizes K and M.

The modal loss factors are calculated by using the undamped mode shapes and the material loss factor for each material. For structures containing a viscoelastic material, the material loss factor of the metal is very small compared to that of the viscoelastic. In this situation the modal loss factor is found from

$$\eta^{(r)} = \eta_{v} \frac{v_{v}^{(r)}}{v_{v}^{(r)}}$$
(4)

where

n = material loss factor of viscoelastic evaluated
v at the r'th calculated resonant frequency

$$\frac{v(r)}{v(r)} = \begin{array}{l} \text{fraction of elastic strain energy attributable} \\ \text{to the viscoelastic when the structure deforms} \\ \text{in the r'th mode shape} \end{array}$$

A derivation of Eq. (4) is given in Section 2.2 of this report. It is shown that modal loss factors obtained from Eq. (4) will approximate those obtained from the complex stiffness eigenvalues of the complementary solution of Eq. (1). However, the modal strain energy approach has the advantage of much lower cost.

Calculation of the modal energy distributions fits quite naturally within finite element methods and is a standard option in some commercial codes. Further advantages of the method are that only undamped normal modes need be calculated and that the energy distributions obtained are of direct use to the designer in deciding where to locate damping material. The disadvantage is that some approximation is required to accommodate the frequency-dependent properties commonly found in viscoelastic materials.

#### 2.2 THEORY

An approximate expression is derived below for the modal loss factor obtained from an eigenvalue analysis of a structure with complex stiffness.

One form of the discretized (i.e., finite element) version of a partial differential equation for free vibration of a structure is:

$$\underbrace{\mathsf{N}}_{\mathbf{X}} + \underbrace{\mathsf{K}}_{\mathbf{X}} = \underbrace{\mathsf{Q}}_{\mathbf{X}} \tag{5}$$

where the stiffness matrix K is constant but complex if the structure contains a viscoelastic material. Equation (5) is converted to an eigenvalue problem by assuming a solution of the form

$$\mathbf{x} = \sum_{\mathbf{r}} \, \phi^{*}(\mathbf{r}) \, e^{ip_{\mathbf{r}}^{*} t}$$
 (6)

where  $p_r^*$  and  $q_r^{*(r)}$  are the r'th complex eigenvalue and eigenvector. That is,

$$\chi^{*(r)} = \chi_{R}^{(r)} + i\chi_{T}^{(r)}$$
(7)

$$p_r^* = p_r (1 + i \eta^{(r)})^{1/2}$$
 (8)

where  $\chi_R^{(r)}$ ,  $\chi_I^{(r)}$ ,  $\eta^{(r)}$ , and  $p_r$  are real. The term  $\eta^{(r)}$  is the loss factor for the r'th mode. The eigenvalue problem is then, from Eqs. (5) and (6):

$$\mathbf{K} \, \mathbf{p}^* = \mathbf{p}^{*2} \, \mathbf{M} \, \mathbf{p}^* \tag{9}$$

Now if K were purely real,  $\phi^{*(r)}$  and  $p_r^*$  would be real and related by the usual Rayleigh's quotient formula:

$$p_{\mathbf{r}}^{2} = \frac{\varrho^{(\mathbf{r})T} \underset{\mathbb{M}}{\times} \varrho^{(\mathbf{r})}}{\varrho^{(\mathbf{r})T} \underset{\mathbb{M}}{\times} \varrho^{(\mathbf{r})}}$$
(10)

where the \* superscript is dropped to denote a real quantity. If K is perturbed by  $\delta K$ , where  $\delta K$  is complex,  $p_r^2$  will likewise acquire an imaginary part which may be written as  $inp^2$  after Eq. (8). Then, if the perturbed stiffness matrix is written as

$$\underline{K} = \underline{K}_{R} + i \underline{K}_{I}$$
(11)

the following is obtained from Eqs. (8), (10), and (11), after dropping the mode index r

$$p^{2}(1+i\eta) = \frac{\ell^{*T} \underbrace{\mathbb{K}_{R} \ell^{*}}}{\ell^{*T} \underbrace{\mathbb{M}} \ell^{*}} + i \frac{\ell^{*T} \underbrace{\mathbb{K}_{I} \ell^{*}}}{\ell^{*T} \underbrace{\mathbb{M}} \ell^{*}}$$
(12)

An approximate value for n can be calculated by approximating the complex eigenvector  $\chi^*$  by the real vector  $\chi$ , calculated from purely elastic analysis, i.e., by suppressing the imaginary part of K. The approach is essentially an extension of Rayleigh's principle into the complex domain. Making this approximation in Eq. (12) and equating real and imaginary parts gives

$$p^{2} = \frac{\phi^{T} \underset{\Phi}{\mathbb{K}}_{R} \psi}{\psi^{T} \underset{\Phi}{\mathbb{M}} \phi}$$
 (13)

$$p^{2}n = \frac{e^{T} \underbrace{K}_{I} e}{e^{T} \underbrace{M}_{\Phi}}$$
 (14)

If the matrix K is obtained by finite element analysis, it may be divided into two additive terms. The first, called  $K_e$ , is obtained from contributions of the purely elastic elements (the metallic portion of the structure). The second, called  $K_v$ , is obtained from the solid elements (used to model the viscoelastic material). Both terms are matrices of the same order as  $K_v$ ,

$$\underline{K} = \underline{K}_{e} + \underline{K}_{v} \tag{15}$$

 $K_e$  will be completely real.  $K_v$  will be complex but, for the present case where only a single viscoelastic material is involved, its imaginary and real parts will have the ratio  $m_v$ :1 where  $m_v$  is the material loss factor of the core. Then,

$$\widetilde{K}_{v} = \widetilde{K}_{vR} + i \widetilde{K}_{vI}$$
(16)

$$= \underbrace{\mathbb{K}}_{\mathbf{v}\mathbf{R}} (1 + i \, \mathbf{n}_{\mathbf{v}}) \tag{17}$$

By previous assumption, only  $\underline{\mathtt{K}}_v$  contributes to  $\underline{\mathtt{K}}_I$  so

$$\widetilde{K}_{I} = \widetilde{K}_{VI}$$
(18)

When a purely elastic normal modes analysis is performed, the strain energy associated with a given mode shape is

$$V = \chi^{T} \underline{K}_{R} \chi \tag{19}$$

The portion of this energy which is attributable to strain in the core is

$$V_{\mathbf{v}} = \mathbf{v}^{\mathrm{T}} \quad \mathbf{K}_{\mathbf{v}\mathrm{R}} \quad \mathbf{v} \tag{20}$$

Eliminating  $p^2$  between Eq. (12) and (13) gives

$$n = n_{V} \frac{2^{T} K_{I} 2}{2^{T} K_{R} 2}$$
 (21)

Combining Eq. (17) through (21) and reinstating the mode index superscript gives the final result for modal loss factor in terms of elastic energies

$$\eta^{(r)} = \eta_{v} \frac{V_{v}^{(r)}}{V_{v}^{(r)}}$$
 (22)

This derivation is intended to motivate and clarify the comparison of results from complex eigenvalue analysis and modal strain energy analysis. It should be noted, however, that the problem statement itself is not entirely realistic. It is well known that complex stiffnesses that do not vary with frequency lead to system responses that are noncausal (response anticipates input) and hence are not physically realizable. Nonetheless, the comparison is believed to be useful in that the symptoms of noncausality are quite weak [4] and the identical assumptions are applied in both methods.

### 2.3 FINITE ELEMENT MODELING OF THREE-LAYER PLATES

#### 2.3.1 Choice of Elements

Modeling of sandwich structures requires that the strain energy due to shearing of the core be accurately represented. Practical considerations dictate that this be done with minimum increase in computation cost relative to a uniform, single-layer model. In this section, a modeling method is described which is reasonably efficient and has the important advantage of being readily implemented within MSC/NASTRAN, a widely available code.

Figure 1 shows the arrangement for modeling of a three-layer The face sheets are modeled with quadrilateral or sandwich. triangular plate elements producing stiffness at two rotational and three translational degrees of freedom per node. The viscoelastic core is modeled with solid elements producing stiffness at three translational degrees of freedom per node. All nodes are at element corners. In MSC/NASTRAN, the plate elements are called TRIA3's, QUAD4's, TRIA6's, and QUAD8's, and the solid elements are called PENTA's and HEXA's. A key feature of these plate elements in the present application is their ability to account for coupling between stretching and bending deformations [5]. This allows the plate nodes to be offset to one surface of the plate, coincident with the corner nodes of the adjoining solid elements. In this way a three-layer plate can be modeled

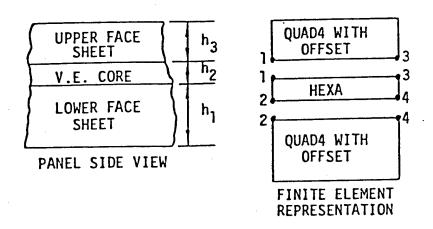


Figure 1 Finite element modeling of a sandwich panel with viscoelastic core

with only two layers of nodes. Earlier methods implemented within NASTRAN were restricted to beams [6] or required four layers of nodes and extensive constraint equations to achieve the proper bending-shearing behavior of the sandwich [7]. Aspect ratios of the solid elements (in-plane dimension/thickness dimension) as high as 5000 have been used successfully to model the thin viscoelastic core layers. In all analyses reported here, Poisson's ratio of the core elements is taken to be 0.49.

# 2.3.2 Reduction of Equations of Motion

In all but the smallest problems, the mass and stiffness matrices are condensed by partitioning and performing a Guyan reduction prior to calculation of eigenvalues. As usual in vibration analysis, some care is warranted in the selection of the degrees of freedom to be retained during this reduction. For the sandwich structures analyzed in this report, only out-ofplane displacements need be retained. Some displacements should be retained for both face sheets, although it is not necessary to keep both upper and lower face displacements at any single location on the model. If out-of-plane displacements of only one face sheet are kept, the results for natural frequencies as well as core-to-total energy ratios can show a pronounced dependence on the Poisson's ratio of the core. Although such a dependence is probably real for some cases, such as doubly curved shells, it should not occur for simpler cases such as straight sandwich beams -- and in fact does not occur if the rule given above is observed in reducing the discretized equations of motion. ing data on Poisson's ratio of most viscoelastic materials are probably not adequate for accurate modeling of doubly curved sandwich shells in the important transition region of the material, and certainly not in the glassy region.

#### 2.3.3 Solution Method

In the modal strain energy method, a standard normal mode extraction run is made with all material constants treated as real and constant. The elastic strain energy in each element for each mode is calculated, as well as the energy fraction in the viscoelastic core for each mode. These fractions multiplied by the core material loss factor give the modal loss factors which are input via a damping vs. frequency table for use in subsequent forced response calculations.

A basic difficulty with the modal strain energy method (or any normal mode method) is that the modal properties are obtained from system matrices that are assumed to be constant. Visco-elastic materials, however, have storage moduli which vary significantly with frequency. There is no theoretically correct way to resolve this contradiction. However, there are great practical advantages to making response predictions in terms of a normal mode set obtained from constant material properties. This can be done with reasonable accuracy if a simple correction is made to the modal loss factors obtained by Equation (4). This correction is only to the modal damping ratios because these are the only modal parameters that can be readily adjusted by the finite element analyst. It is explained here for completeness even though no forced response calculations were performed for this report. The correction is obtained as follows.

For broadband excitation, most of the response of a given mode occurs within a narrow band around the mode's natural frequency. It is natural then to require that the energy distribution used to compute the loss factor for a given mode be obtained using a stiffness matrix evaluated for material properties taken at that mode's frequency. Because the natural frequencies themselves depend on material properties, an iterative solution of two simultaneous relations (the eigenvalue problem for each mode number and the material property vs. frequency relation) is required. This is readily done [1], but a further problem remains. The final modal coordinate representation of the structure must

come from a single stiffness matrix evaluated using a single value of storage modulus for the core material. Natural frequencies, mode shapes, and modal masses will be correct for, at most, one mode. A further correction of the modal loss factor has been found to give some improvement.

Each modal equation of motion has the form given in Equation (2). At resonance the first and last terms on the left cancel each other. The response magnitude is inversely proportional to the product  $n^{(r)}\omega_r$  which is the coefficient of  $\dot{\alpha}_r$ , the modal velocity. If  $n^{(r)}$  is altered to correct for the error in  $\omega_r$ , an improvement in peak response can be expected, although resonance will still occur at a slightly shifted frequency and some error will remain due to  $\ell_r$  which depends on modal mass. In test cases run for sandwich beams [1], it was found that taking  $\omega_r$  to be proportional to  $\sqrt{G_2}$  ( $G_2$  = core shear modulus) would improve the agreement between the MSE method and the direct frequency response method. This is of course an approximation since  $\omega_r$  depends on properties of the face sheets as well as the core. The modal damping ratios are adjusted according to

$$\eta^{(r)} = \eta^{(r)} \qquad \sqrt{\frac{G_2(f_r)}{G_{2,ref}}}$$
 (23)

where

 $\eta(r)$  = adjusted modal damping ratio for the r'th mode

 $\eta^{(r)}$  = modal damping ratio for the r'th mode obtained by iteration

G<sub>2,ref</sub> = core shear modulus used in final normal modes calculation to obtain modal frequencies, shapes, and masses

 $^{G}$ 2( $f_{r}$ ) = core shear modulus at  $f = f_{r}$  where  $f_{r}$  is r'th mode frequency calculated with  $^{G}$ 2 =  $^{G}$ 2.ref

#### 2.4 EXAMPLE

A closed form solution exists for the complex eigenvalues (i.e., natural frequencies and modal loss factors) of a simply supported sandwich plate [8]. The solution is described in Section 4.0 of this report in connection with the design of constrained-layer damping treatments for high-order local modes of plate sections. In this section it is used to verify one of the most important implications of the MSE method; namely, that the modal loss factors for all modes of a sandwich plate are directly proportional to the material loss factor of the viscoelastic core. The sample problem is also used to illustrate the input data to NASTRAN for MSE analysis of a sandwich plate.

Figures 2 through 5 give a comparison of modal loss factors obtained by using MSC/NASTRAN and MSE (MSC/NASTRAN-MSE) and by the closed form solution of Ref. [8]. The format of these plots is used throughout this report and is explained in detail in Section 3.1. In brief, the ordinate is a dimensionless quantity proportional to modal loss factor and the abscissa is a dimensionless quantity proportional to the shear modulus of the viscoelastic core. The curves marked with specific values of  $\mathbf{n}_{\mathbf{V}}$  are obtained from the closed form solution. The remaining curve, obtained by the MSC/NASTRAN-MSE method, gives results that are inherently independent of  $\mathbf{n}_{\mathbf{V}}$ .

The test case is a simply supported rectangular sandwich plate of the following dimensions:

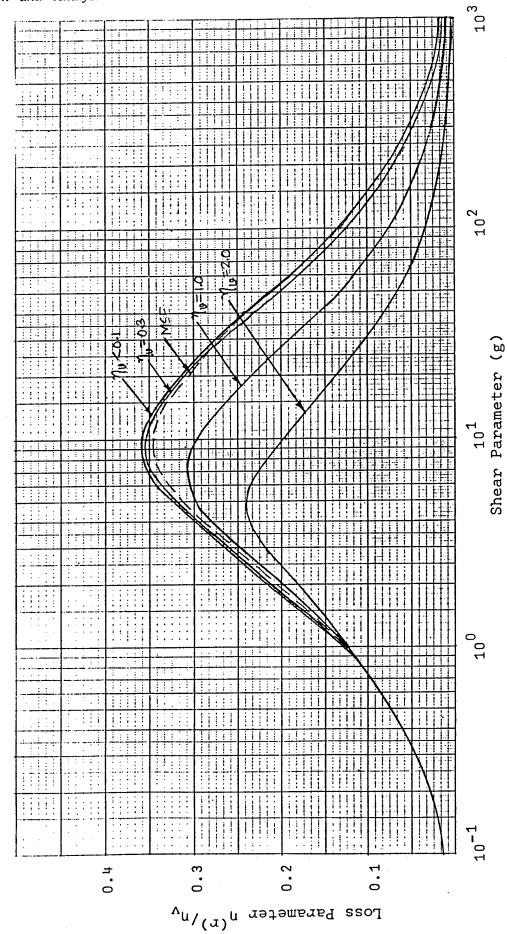
in-plane dimensions = 10" x 11" upper face sheet thickness = 0.055" lower face sheet thickness = 0.055" core thickness = 0.0045" = aluminum  $E = 10^7 \text{ psi}$   $\rho = 0.1 \text{ lb/in}^3$   $\nu = 0.3$ 

shear modulus of core material = variable loss factor of the core material = variable

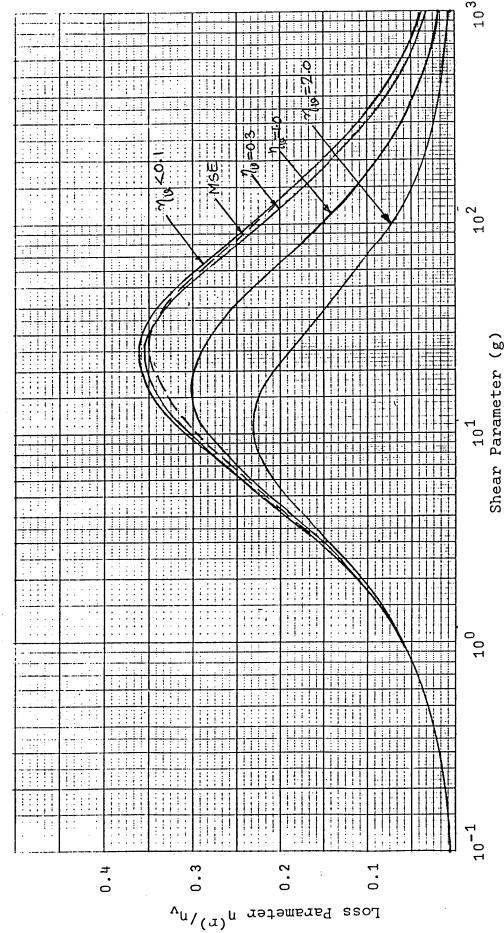
Figures 2 through 5 show that the closed form and MSC/NASTRAN-MSE results agree very closely for small values of the material loss factor. Some divergence is seen for larger values on the order of unity or greater. The agreement also depends on the value of the shear parameter g. It is best for g equal to or less than the value giving highest damping. Fortunately, most practical constrained layer treatments tend to fall in this range.

A tabular representation of the closed form results used to prepare Figures 2 through 5 is given in Tables 1 through 4. Results for higher modes are also given in the tables.

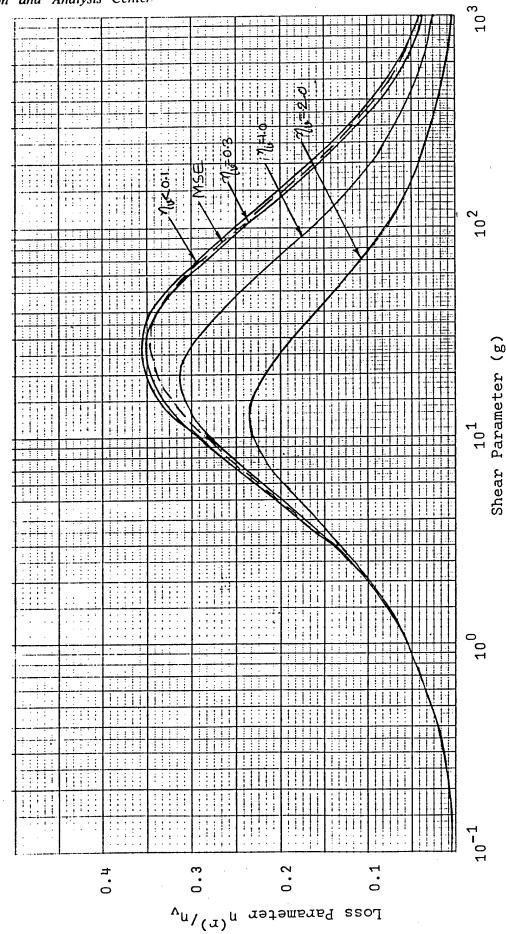
Sample NASTRAN input and output are given in Appendix A for a plate with the properties listed above and a core shear modulus of 450 psi. This sample case corresponds to a dimensionless shear parameter of g = 40.



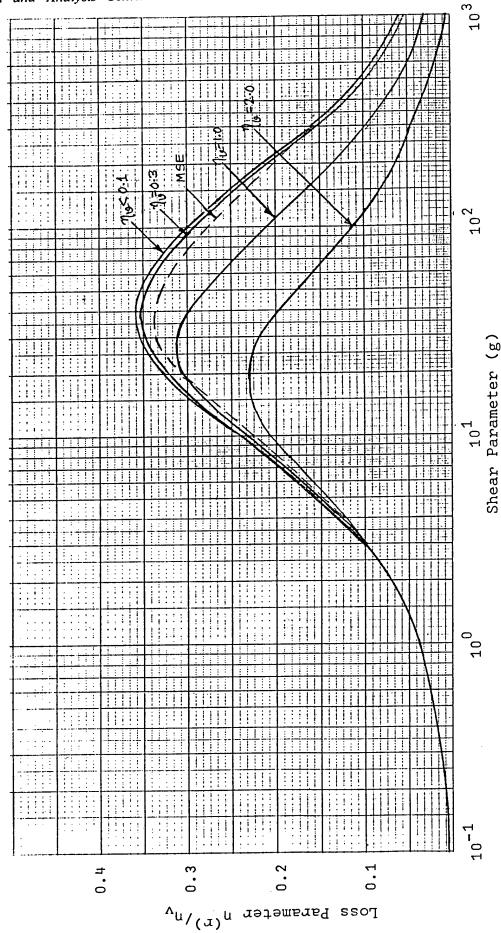
rectangular sandwich method and by method Damping of the first mode of simply supported plate obtained by NASTRAN/Modal Strain Energy exact complex eigenvalue solution [8] ~



s second mode of simply supported rectangular sandwich by NASTRAN/Modal Strain Energy method and by eigenvalue solution [8] Damping of the splate obtained be exact complex ei က Figure



Damping of the third mode of simply supported rectangular sandwich plate obtained by NASTRAN/Modal Strain Energy method and by exact complex eigenvalue solution [8] ⇉



Damping of the fourth mode of simply supported rectangular sandwich plate obtained by NASTRAN/Modal Strain Energy method and by exact complex eigenvalue solution [8] Ŋ Figure

TABLE 1 MODAL FREQUENCIES AND MODAL LOSS FACTORS FOR A RECTANGULAR SANDWICH PLATE

Aspect Ratio ( $\Delta xy$ ) = 1. Geometric Parameter (Y) = 3. Viscoelastic Loss Factor ( $n_V$ ) = 0.

						SHEAR F	SHEAR PARAMETER (g)	(g)				
		0.1	Steel F	Face Sheets		6.0	30		luminum 1	Aluminum Face Sheets 200.	1000	.00
MODE (r)	Freg.	Loss Parameter	Freq.	Loss Parameter	Freq.	Loss Parameter	Freq.	Loss Parameter	Freq.	Loss Parameter	Freq.	Loss
	- 1	(11)	( r /	(II)	(1,1)	(h)	(T <sub>r</sub> )	(H)	(۲٫)	('n)	(†,	(ת)
<b>,</b>	530.	0.0016	565.	0.0128	698.	0.0338	163.	0.0281	190.	0.0074	196.	0.0016
2	1244.	0.0007	1280.	0.0062	1450.	0.0241	333.	0.0355	428.	0.0149	458.	0.0037
က	1394.	0.0006	1430.	0.0056	1604.	0.0226	366.	0.0358	476.	0.0162	512.	0.0041
4	2107.	0.0004	2145.	0.0038	2329.	0.0173	514.	0.0352	493.	0.0214	767.	0.0061
ഹ	2433.	0.0004	2471.	0.0033	2658.	0.0156	579.	0.0343	787.	0.0233	882.	0.0069
9	2833.	0.0003	2870.	0.0029	3061.	0.0139	657.	0.0332	900.	0.0253	1021.	0.0079
7	3297.	0.0003	3334.	0.0025	3528.	0.0124	746.	0.0317	1026.	0.0273	1181.	0.0090
<b>&amp;</b>	3547.	0.0002	3584.	0.0023	3779.	0.0116	793.	0.0310	1092.	0.0282	1266.	9600.0
6	4099.	0.0002	4136.	0.0020	4333.	0.0104	897.	0.0293	1237.	0.0299	1453.	0.0108
10	4736.	0.0002	4774.	0.0017	4973.	0.0092	1016.	0.0275	1398.	0.0315	1665.	0.0121

TABLE 2
MODAL FREQUENCIES AND MODAL LOSS FACTORS
FOR A RECTANGULAR SANDWICH PLATE

Aspect Ratio ( $\Delta xy$ ) = 1.1 Geometric Parameter (Y) = 3.5 Viscoelastic Loss Factor ( $n_V$ ) = 0.3

						SHEAR P	SHEAR PARAMETER (g)	(g) ×				
	0	1	Steel Fac 1.0	Face Sheets 1.0		6.0	30		uminum Fa	Aluminum Face Sheets 200.	ts 1000	0.
MODE (r)	Freq. Pa	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq. (f,)	Loss Parameter (n)	Freq.	Loss rameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss Parameter (n)
-	530.	0.005	565.	0.038	700.	0.101	172.	0.188	191.	0.021	196.	0.005
2	1244.	0.002	1280.	1280. 0.018	1451.	0.072	352.	0.281	430.	0.042	458.	0.010
က	1394.	0.002	1430.	1430. 0.017	1605.	0.067	385.	0.289	478.	0.046	513.	0.012
4	2107.	0.001	2145.	2145. 0.011	2330.	0.052	535.	0.305	.969	0.061	769.	0.017
ம	2433.	0.001	2471.	2471. 0.010	2659.	0.047	.009	0.304	792.	0.067	884.	0.019
9	2833.	0.001	2870.	2870. 0.086	3062.	0.042	678.	0.299	905.	0.073	1024.	0.022
	3297.	0.001	3335.	3335. 0.074	3529.	0.037	766.	0.292	1033.	0.079	1184.	0.025
∞	3547.	0.001	3584.	3584. 0.069	3780.	0.035	813.	0.287	1100.	0.081	1270.	0.027
6	4099.	0.001	4136.	4136. 0.006	4334.	0.031	916.	0.275	1245.	0.087	1458.	0.030
01	4736.	0.001	4774.	4774. 0.005	4973.	0.027	1034.	0.262	1406.	0.092	1671.	0.034

TABLE 3
MODAL FREQUENCIES AND MODAL LOSS FACTORS
FOR A RECTANGULAR SANDWICH PLATE

Aspect Ratio ( $\Delta xy$ ) = 1.3 Geometric Parameter (Y) = 3.9 Viscoelastic Loss Factor ( $n_{\chi}$ ) = 1.0

						SHEAR F	SHEAR PARAMETER (g)	(6) s				
		0.1	Steel F	Steel Face Sheets 1.0		6.	30		Aluminum Face 200.	Face Sheets		1000.
MODE (r)	Freq. (f,)	Loss Para <u>m</u> eter (n)	Freq. (f,)	Loss Para <u>meter</u> (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss arameter (n)	Freq.	Loss arameter (n)	Freq.	Loss Parameter (n)
_	530.	530. 0.016	566.	0.127	721.	0.302	172.	0.188	194.	0.040	197.	0.008
2	1244.	0.007	1281.	0.062	1467.	0.232	352.	0.281	443.	980.0	462.	0.019
ო	1394.	900.0	1431.	0.056	1620.	0.219	385.	0.289	494.	0.095	517.	0.022
4	2107.	0.004	2145.	0.038	2342.	0.170	535.	0.305	726.	0132.	778.	0.032
ഹ	2433.	0.003	2471.	0.033	2670.	0.154	.009	0.304	828.	0.147	897.	0.037
9	2833.	0.003	2871.	0.029	3072.	0.138	678.	0.299	949.	0.163	1041.	0.043
7	3297.	0.003	3335.	0.025	3537.	0.123	766.	0.292	1085.	0.181	1207.	0.049
∞	3547.	0.002	3585.	0.023	3788.	0.116	813.	0.287	1157.	0.189	1295.	0.052
6	4099.	0.002	4137.	0.020	4341.	0.103	916.	0.275	1311.	0.207	1490.	090.0
9	4736.	0.002	4774.	0.017	4980.	0.091	1035.	0.2615	1483.	0.224	1713.	0.068

TABLE 4
MODAL FREQUENCIES AND MODAL LOSS FACTO
FOR A RECTANGILLAR SANDMICH PLATE

Aspect Ratio  $(\Delta xy)$  = 1.1 Geometric Parameter (Y) = 3.5 Viscoelastic Loss Factor  $(n_V)$  = 2.0

<b></b>						SHEAR P	SHEAR PARAMETER (9)	R (g)				
	J	0.1	Steel Fa	Face Sheets 1.		6.	30.		Aluminum Face 200.	Face Sheets 200.	1000	0.
	Freg. P (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Para <u>m</u> eter (n)	Freq. (f,)	Loss Parameter (n)	Freq. (f,)	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter (n)
	530.	0.031	571.	0.248	778.	0.458	184.	0.188	196.	0.033	198.	0.003
	1244.	0.014	1283.	0.123	1518.	0.421	385.	0.344	456.	0.075	465.	0.008
	1394.	0.012	1433.	0.111	1668.	0.403	422.	0.366	510.	0.083	521.	0.009
	2107.	0.008	2146.	920.0	2379.	0.326	583.	0.434	759.	0.122	786.	0.013
	2433.	0.007	2472.	990.0	2704.	0.297	650.	0.451	870.	0.138	907.	0.0152
	2833.	900.0	2872.	0.057	3102.	0.268	729.	0.462	1004.	0.158	1055.	0.018
	3297.	0.005	3336.	0.049	3564.	0.240	818.	0.468	1156.	0.179	1225.	0.021
	3547.	0.005	3585.	0.046	3813.	0.228	865.	0.468	1236.	0.190	1317.	0.044
	4099.	0.004	4136.	0.040	4364.	0.203	. 296	0.463	1409.	0.213	1519.	0.051
	4736.	0.004	4775.	0.035	5000.	0.181	1083.	0.453	1603.	0.238	1751.	0.058

#### 3.0 DESIGN CHARTS FOR SANDWICH PLATES

In this section, sets of design charts are given which allow a user to rapidly estimate the loss factors and natural frequencies for a rectangular sandwich plate of various boundary conditions. The charts were compiled from a large number of NASTRAN analyses using the modal strain energy method. They allow fairly accurate predictions of damping which take boundary conditions into account and yet do not require the user to actually prepare or run any finite element models. The usefulness of these charts derives from the fact that, to the authors' knowledge, no exact solutions exist for other than simply supported boundary conditions.

#### 3.1 DATA FORMAT

The charts are in terms of dimensionless variables in order to convey the maximum amount of information. It may be shown [9] that a rectangular sandwich plate can be completely described by four dimensionless parameters:

$$\eta_v$$
 = core material loss factor

g = shear parameter

$$= \frac{\overline{G}}{T_2} \left( \frac{1}{E_1 T_1} + \frac{1}{E_3 T_3} \right) a^2 (1 - v^2)$$
 (24)

Y = geometry parameter

$$=\frac{\left(T_1+T_3+2T_2\right)^2}{4D(1-v^2)}\left[\frac{E_1T_1E_3T_3}{E_1T_1+E_3T_3}\right] \tag{25}$$

$$\Delta xy = in-plane aspect ratio$$
 (26)  
=  $b/a$ 

where

 $T_1, T_3$  = thicknesses of the face sheets

 $T_2$  = thickness of the core layer

 $\overline{G}$  = real part of the complex shear modulus  $\left[\overline{G}(1+i\eta_v)\right]$  of the viscoelastic material

 $E_1, E_3$  = Young's moduli of the face sheets

a,b = in-plane dimensions of the plate

D = sum of the flexural stiffnesses of the upper and lower face sheets, each about its own center plane

v = Poisson's ratio of the face sheets

The charts give the dimensionless loss parameter,  $\eta^{(r)}/\eta_v$ , as a function of the shear parameter for the first four bending modes of a plate for various values of the aspect ratio and geometry parameter. A value of approximately Y = 3.5 characterizes a sandwich plate with equal thickness face sheets. The situation of equal face sheets is fairly common in practice and therefore is included for all boundary conditions. Additional values of Y are included for some boundary conditions to cover the case of unequal face sheets which often occurs with add-on constrained layer damping treatments.

Natural frequencies, in a normalized form, are also given as a function of shear parameter for the first four modes. The form is  $f_r/f_{01}$  where  $f_r$  is the natural frequency of the r'th mode. The reference frequency  $f_{01}$  is defined as the first natural frequency of a simply supported plate of the same in-plane dimensions as the actual plate but with flexural stiffness equal to the sum of the stiffnesses of the upper and lower face sheets. They are calculated using the formula [9]:

$$f_{O1} = \frac{1}{2\pi} \sqrt{\frac{D}{\rho} \left[ \left( \frac{m\pi}{a} \right)^2 + \left( \frac{n\pi}{b} \right)^2 \right]^2}$$
; m=1 n=1 (27)

where

 $\rho$  = mass density per unit area of the plate

The reference frequencies calculated by Eq. (27) are given in Table 5. It may be noted that they are not strictly functions of only the shear and geometry parameters but also depend on the material properties of the face sheets. Since two different sets of properties (corresponding to steel and aluminum) were used to span the desired range of the shear parameter, the reference frequencies corresponding to each are given.

Summary tables of the NASTRAN results used to prepare the frequency and damping plots are given for each set of boundary conditions. The tables give results for the fifth and higher modes, in addition to the first four modes for which results are plotted.

Each graph and table is marked with a three letter abbreviation denoting the boundary conditions. The first character in every case is a P (pinned) and indicates that out-of-plane displacements of both face sheets were constrained. The second character is either a T (tilting), L (level), or W (wind-up). Respectively, they designate an unconstrained, perfectly constrained, or elastically constrained condition on rotation of the face sheets about an axis parallel to the plate edge. The third character is either a U (unriveted) or an R (riveted). Riveted implies that shearing deformation of the core has been constrained along the plate edge.

TABLE 5 REFERENCE FREQUENCIES

		S			
GEOMETRIC PARAMETER (Y)	4.5	Steel Face Sheets	351.	794.	2700.
		Aluminum Face Sheets	78./57.*	177./129.*	601./438.*
	3.5	Steel Face Sheets	527.	1193.	4055.
		Aluminum Face Sheets	94.	212.	722.
	1.5	Steel Face Sheets	468.	1058.	3599.
		Aluminum Face Sheets	76.	171.	582.
	0.5	Aluminum Steel Face Sheets Face Sheets	838.	1894.	6437.
		Aluminum Face Sheets	81.	183.	622.
ASPECT RATIO			$\Delta xy = 1.1$ a = 10.0 b = 11.0	Δxy = 2.0 a = 5.5 b = 11.0	$\Delta xy = 4.0$ b = 2.75 a = 11.0

\* for shear parameter = 1000.

## 3.2 DESIGN CHARTS

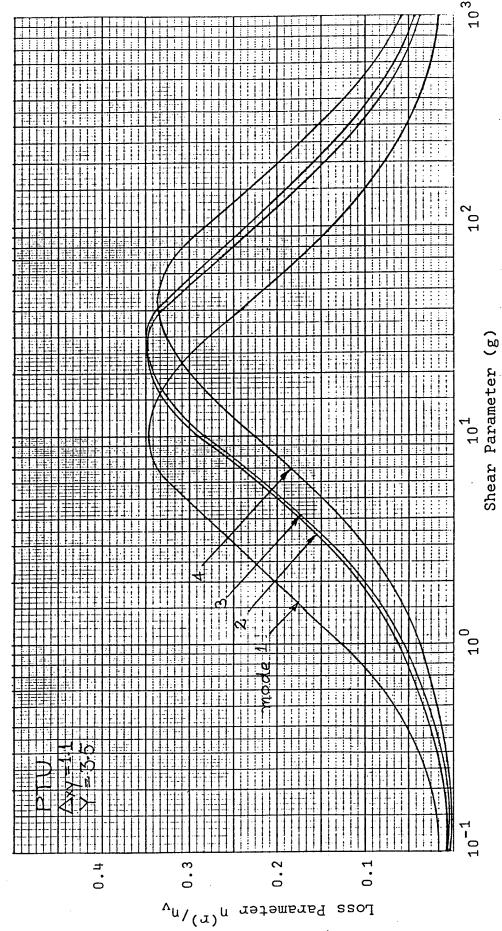
## 3.2.1 PTU Boundary Conditions

The results for PTU (simply-supported, unriveted) boundary conditions are given in Figures 6 through 14 and Tables 6 through 17. These boundary conditions are likely to be appropriate for plate sections in lightweight, built-up structures using add-on constrained layer damping. Several values of the geometry parameter are used since it is likely to be a design variable in these, situations.

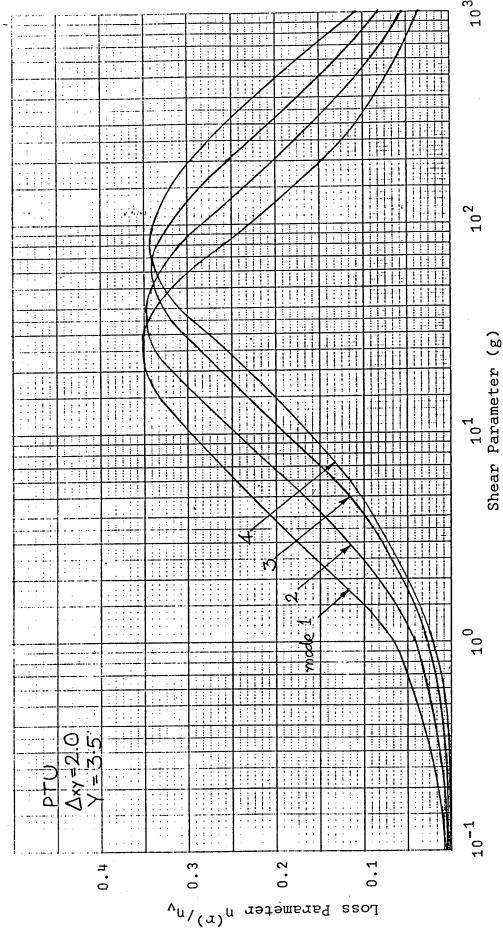
Figure 6 shows damping as a function of shear parameter for the first four modes, with a geometry parameter of Y=3.5 and an aspect ratio  $\Delta xy=1.1$ . Figures 7 and 8 show similar information for aspect ratios of 2.0 and 4.0. The next six figures, 9 through 14, give similar data but organized to show how damping varies with the geometric parameter as well as the shear parameter. For clarity, the data covering four modes (for each value of aspect ratio) is split into two plots, with each plot covering two alternate modes.

Natural frequencies of sandwich plates with PTU boundary conditions can be obtained from Figures 15 through 26. Each plot gives results for one of three aspect ratios (1.1, 2.0, or 4.0) and one of four geometry parameters (0.5, 1.5, 3.5, or 4.5). Reference frequencies used in normalizing the data of these figures are given in Table 5.

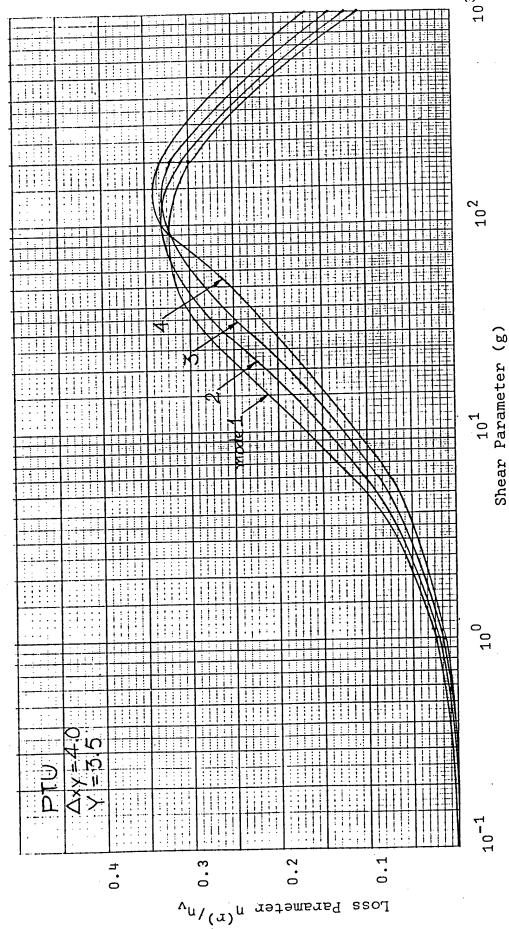
A tabular representation of the data in Figures 6 through 26 as well as data for higher modes is given in Tables 6 through 17.



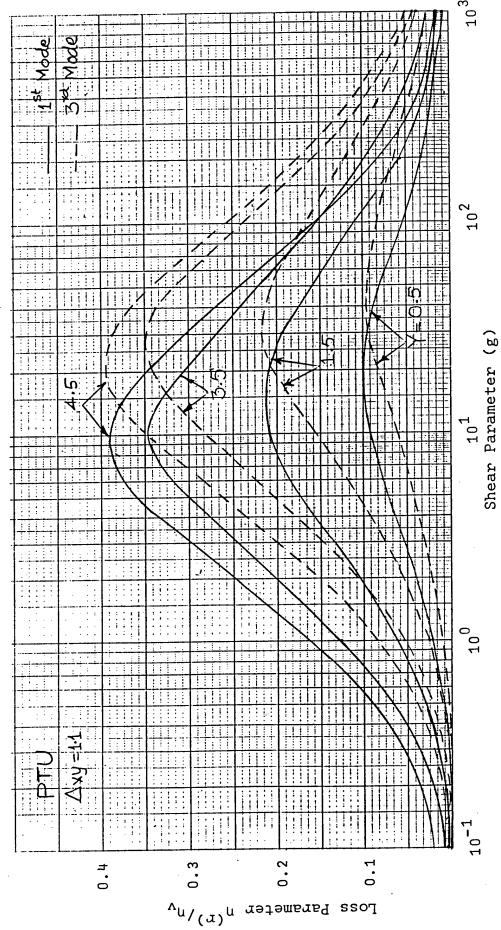
sandwich rectangular plate, PTU boundary conditions,
= 3.5



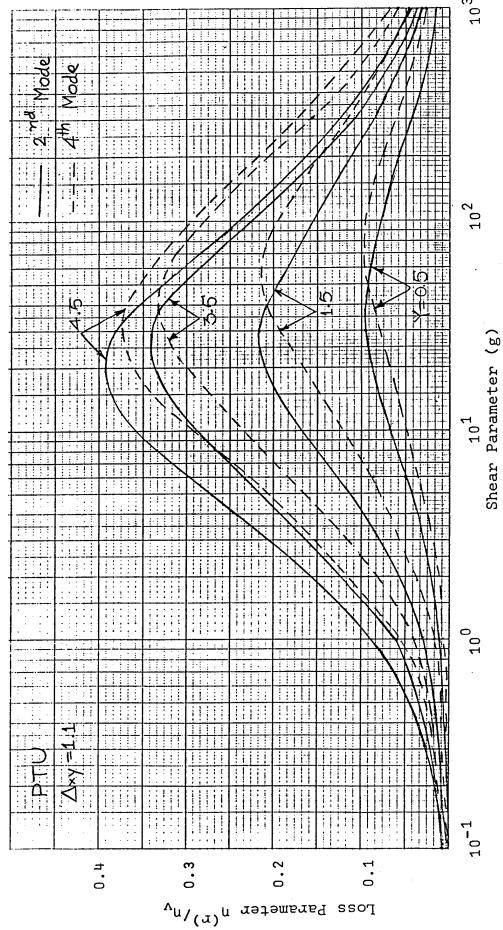
PTU boundary conditions, sandwich rectangular plate, = 3.5 of 0, Damping  $\Delta xy = 2$ .



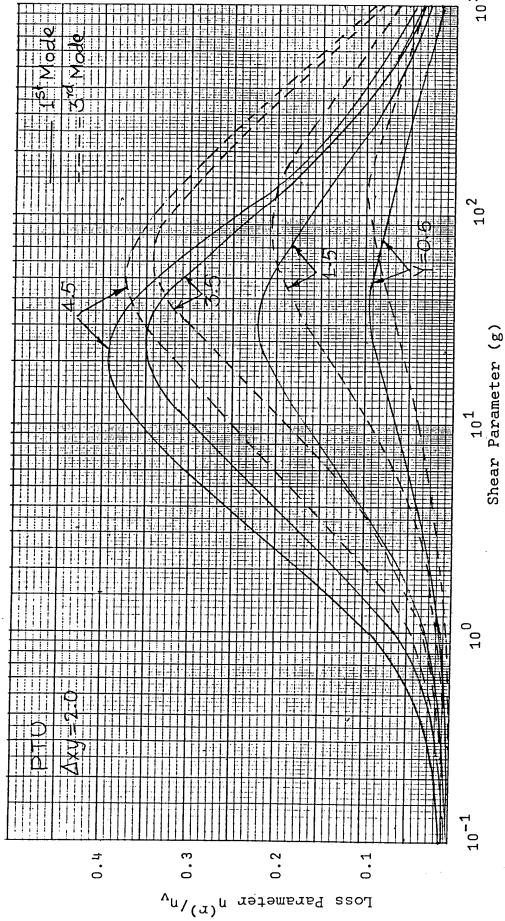
sandwich rectangular plate, PTU boundary conditions = 3.5 ದ ≻ 0 to Damping  $\Delta xy$ ω



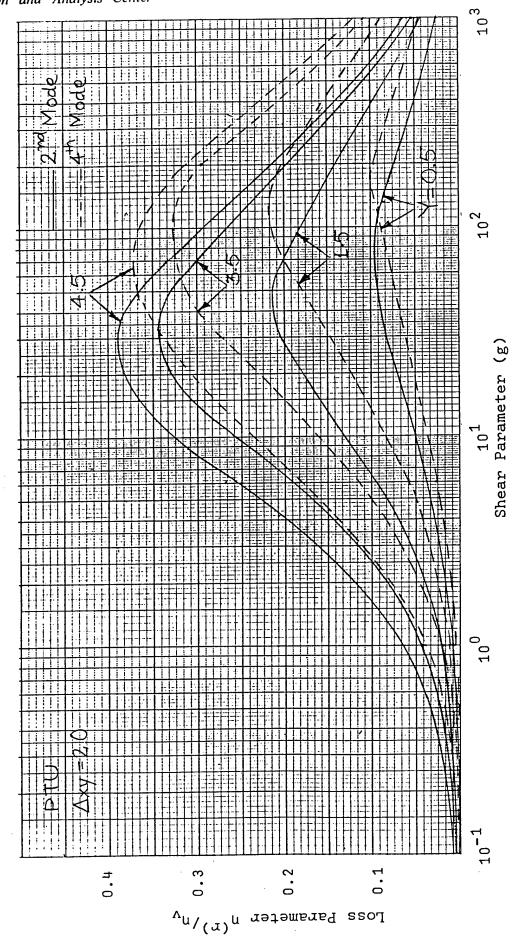
PTU boundary conditions g and sandwich rectangular plate, 3, variable Y modes 1 and ಥ of 1, Damping  $\triangle xy = 1.$ σ



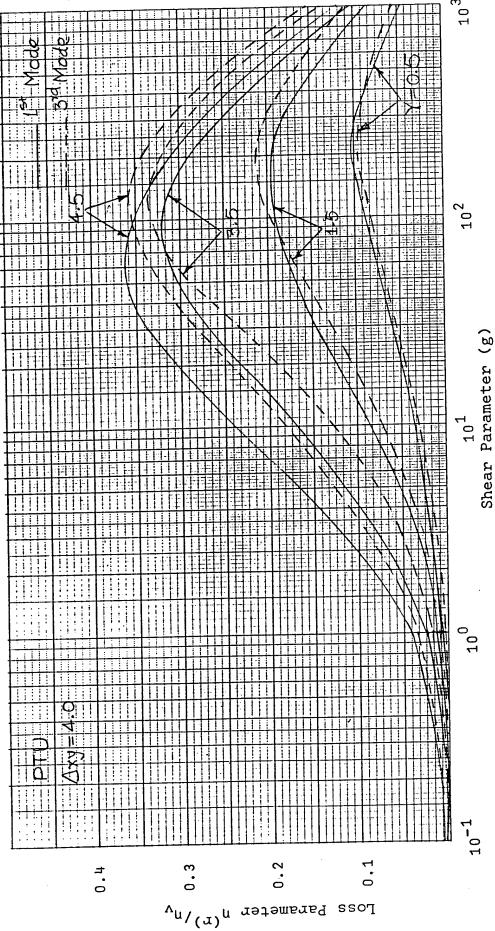
PTU boundary conditions sandwich rectangular plate, I nodes 2 and 4, variable Y and of [.1,



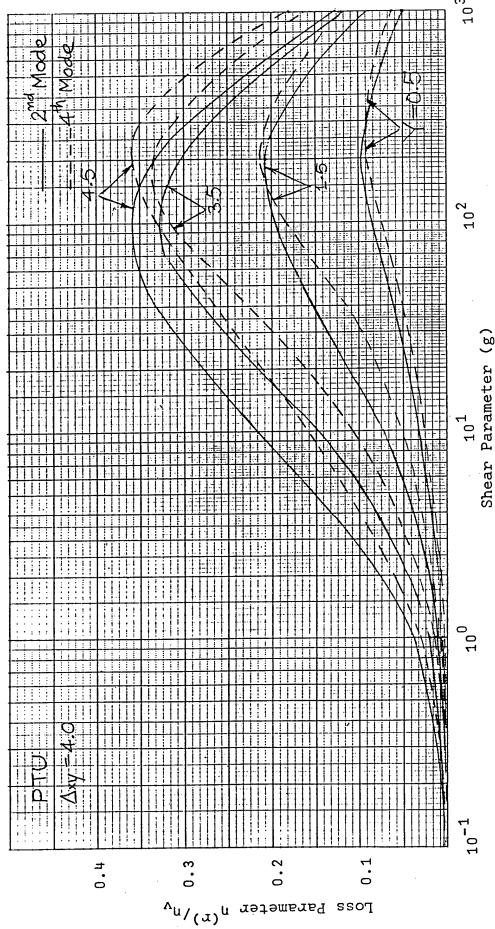
PTU boundary conditions, a sandwich rectangular plate, modes 1 and 3, variable Y and Damping of  $\Delta xy = 2.0$ , IJ



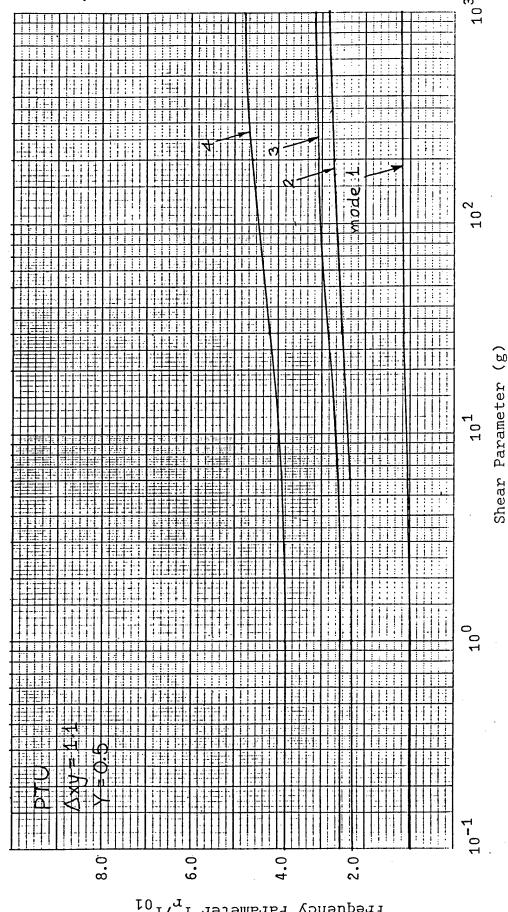
a sandwich rectangular plate, modes 2 and  $\mbox{\ensuremath{\mu}}$  , variable Y and Damping of  $\Delta xy = 2.0$ , Figure 12



PTU boundary conditions g a sandwich rectangular plate, modes 1 and 3, variable Y and Damping of  $\Delta xy = 4.0$ , 13



PTU boundary conditions, g a sandwich rectangular plate, modes 2 and  $\mu$ , variable Y and Damping of  $\Delta xy = 4.0$ ,



rectangular

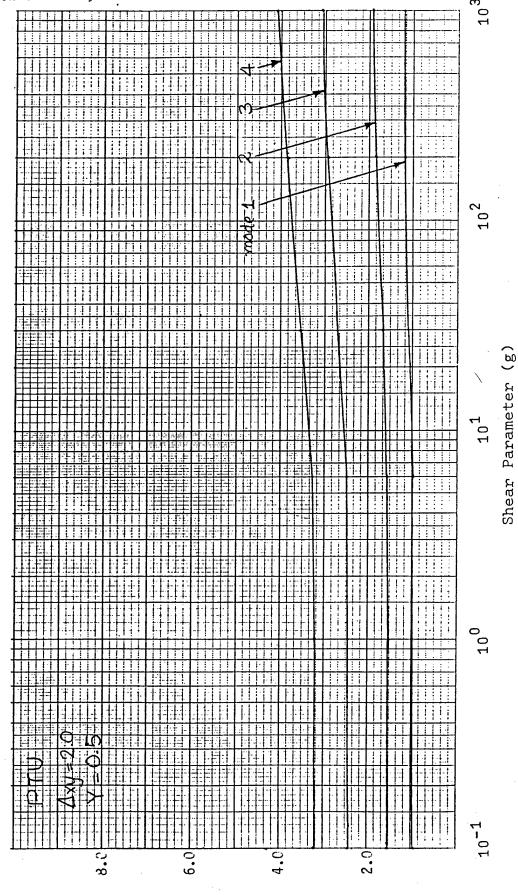
sandwich

н

 $\Delta xy$ 

Natural frequencies of sa PTU boundary conditions,

Erequency Parameter  $f_{\rm r}/f_{\rm 01}$ 

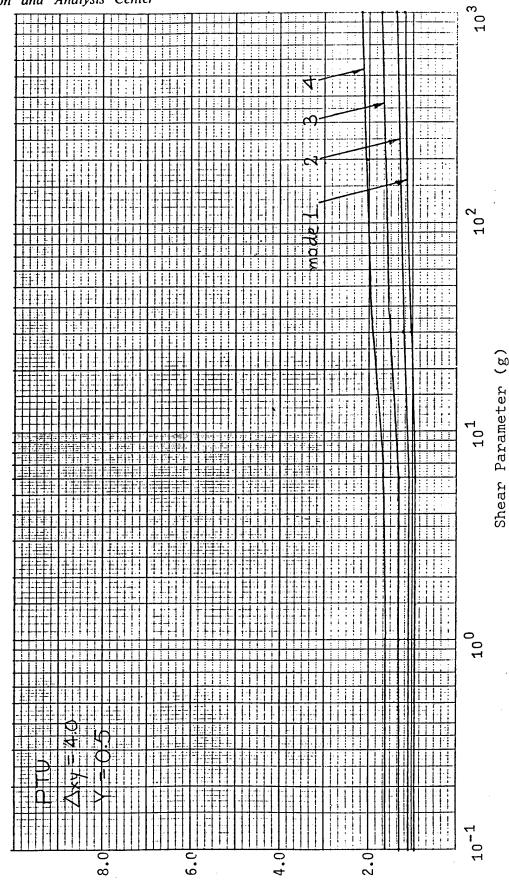


sandwich rectangular,  $\Delta xy = 2.0$ , Y = 0.5

Natural frequencies of sandwic PTU boundary condition,  $\Delta xy =$ 

16

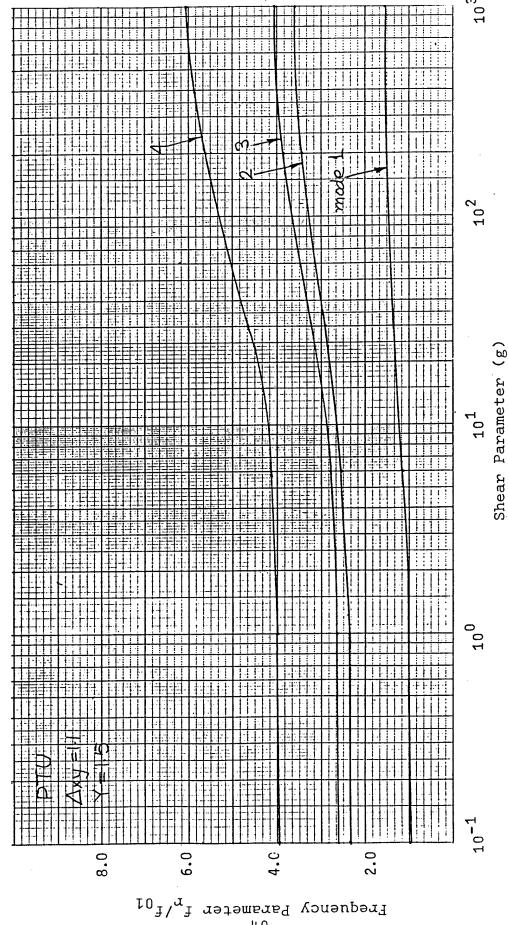
Erequency Parameter  $f_{\nu}/f_{01}$ 



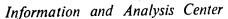
Natural frequencies of sandwich rectangular PTU boundary conditions,  $\Delta xy = 4.0$ , Y = 0.5

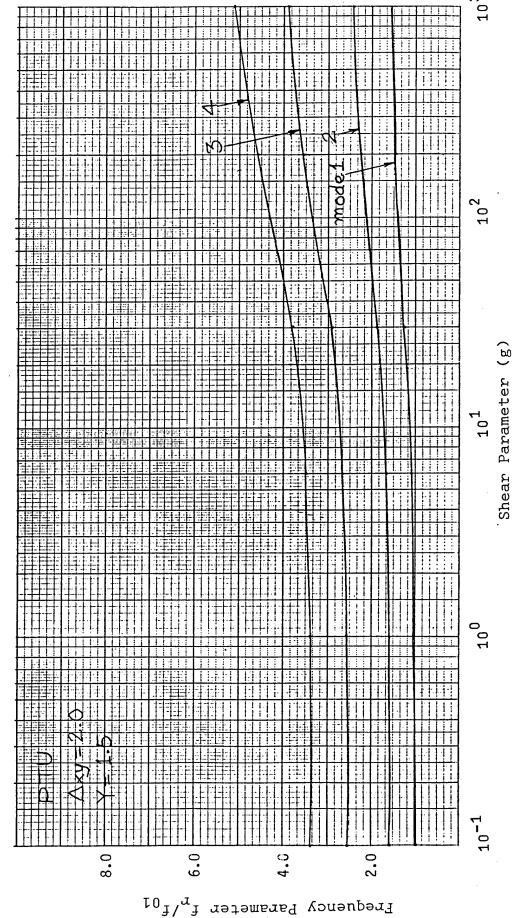
17

Leedneuch barameter  $t^{L}/t^{01}$ 

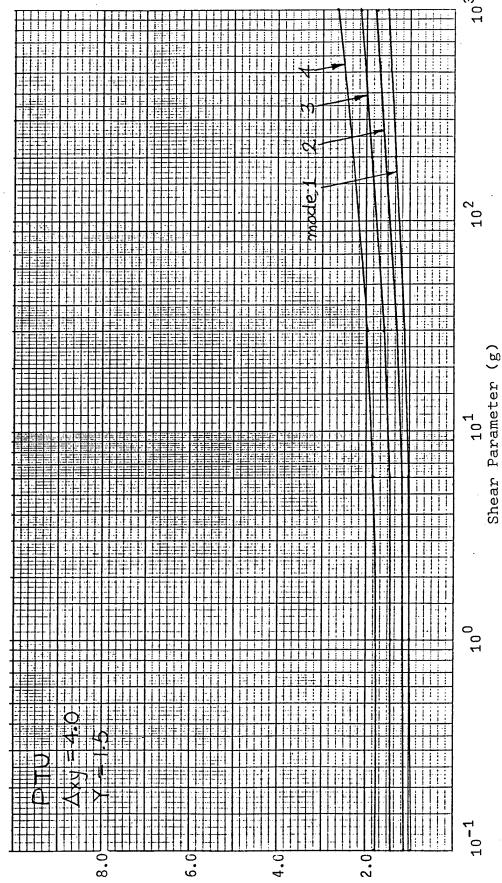


plate, Natural frequencies of sandwich rectangular PTU boundary conditions,  $\Delta xy = 1.1$ , Y = 1.5





plate, rectangular 2.0, Y = 1.5 sandwich conditions, Natural frequencies of PTU boundary conditions 13

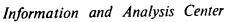


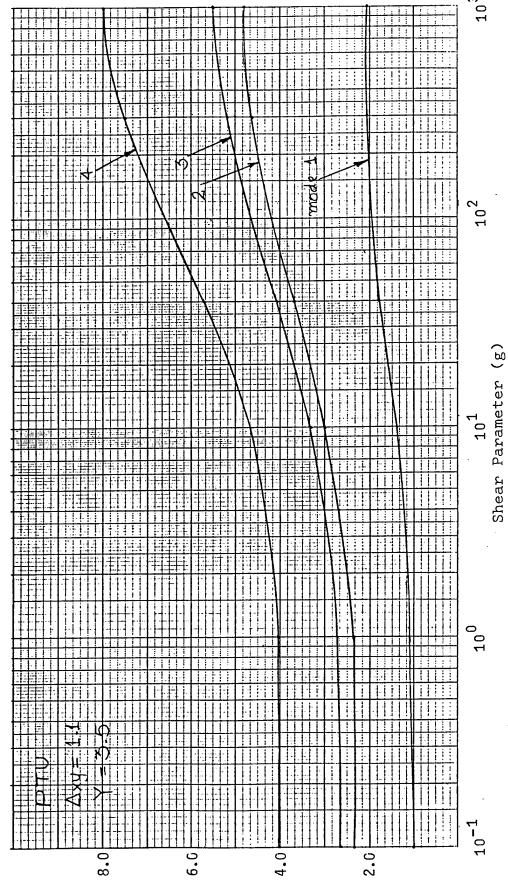
Natural frequencies of sandwich rectangular PTU boundary conditions,  $\Delta xy = 4.0$ , Y = 1.5

20

Figure

Frequency Parameter f<sub>r</sub>/f<sub>01</sub>



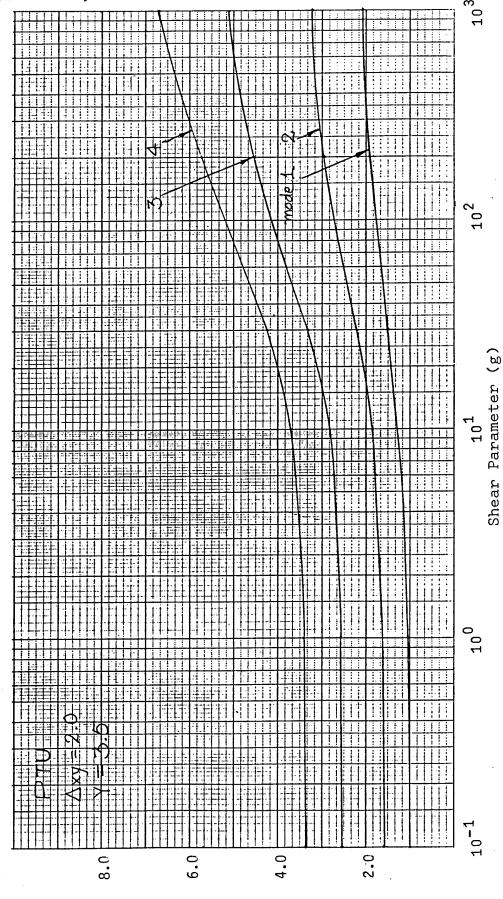


plate, Y = 3.

sandwich xy = 1.1,

Natural frequencies of saboundary conditions,  $\Delta xy$ 

Leedneuch barameter fr/f01



rectangular

2.0,

11

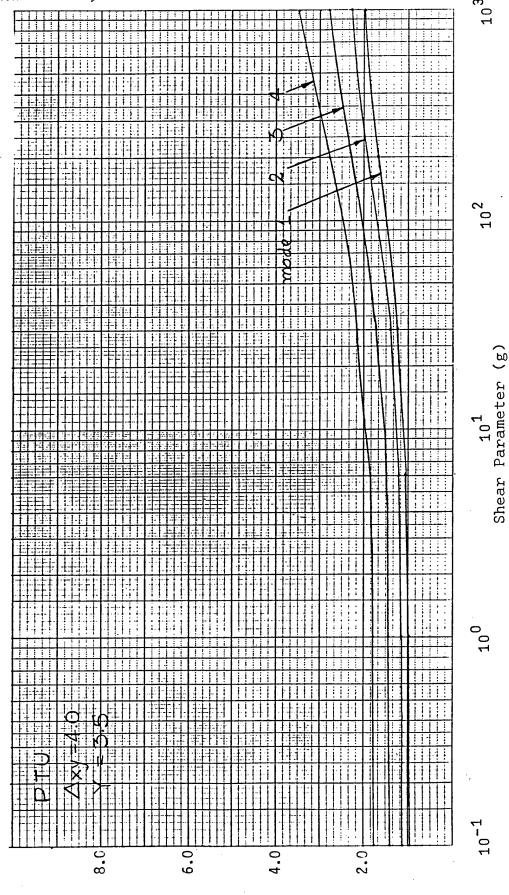
 $\Delta xy$ 

Natural frequencies of sa PTU boundary conditions,

22

sandwich

Frequency Parameter  $f_{\rm r}/f_{\rm 01}$ 

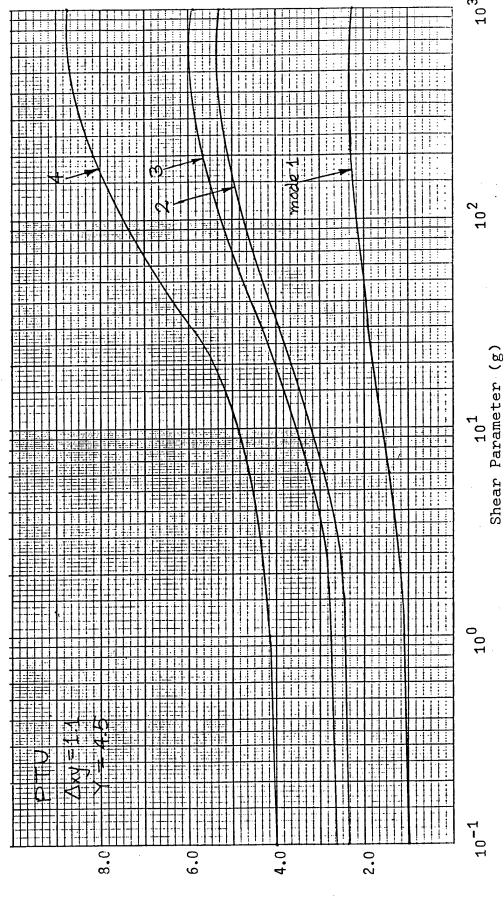


Natural frequencies of sandwich rectangular PTU boundary conditions,  $\Delta xy = 4.0$ , Y = 3.5

23

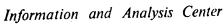
Figure

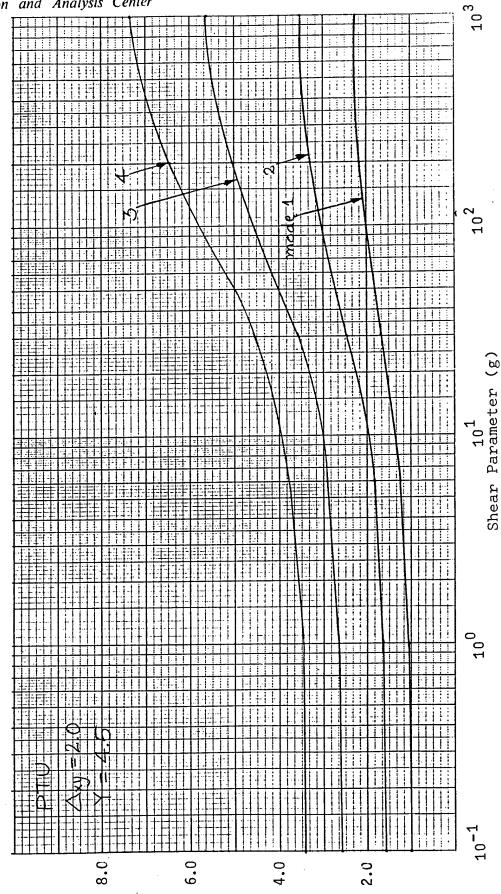
Frequency Parameter f<sub>r</sub>/f<sub>01</sub>



Natural frequencies of sandwich rectangular PTU boundary conditions,  $\Delta xy = 1.1$ , Y = 4.5

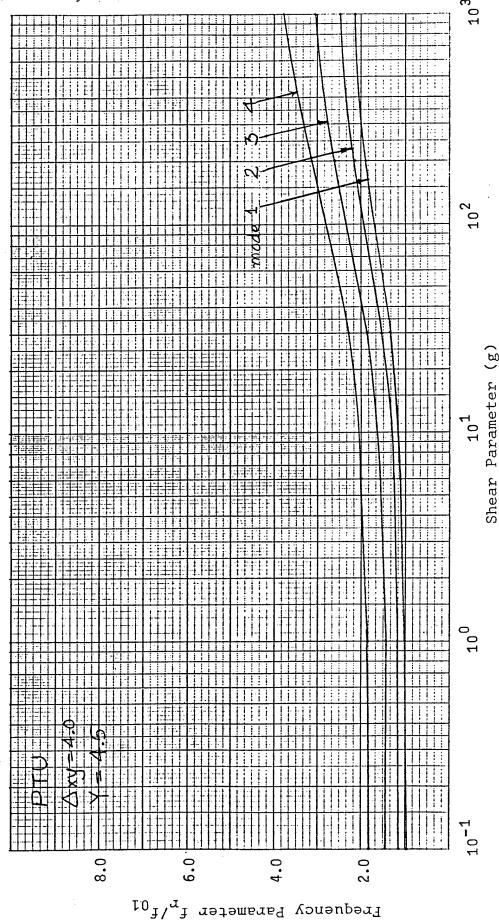
Erequency Parameter fr/f<sub>01</sub>





Natural frequencies of sandwich rectangular PTU boundary conditions,  $\Delta xy = 2.0$ , Y = 4.5

Lredneucy Parameter fr/f<sub>01</sub>



Natural frequencies of sandwich rectangular plate, PTU boundary conditions,  $\Delta xy = 4.0$ , Y = 4.526

TABLE 6 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, unrestrained rotation, unrestrained shear) PTU 1.1 0.5 11 11 11 Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (Y)

						SHEAR PARAMETER (9)	ARAMET	ER (g)				
		0.1	Steel	Steel Face Sheets 1.0		6.0		30.	Aluminum Face 200.	m Face Sheets 200.		1000
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freg. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f,)	Loss Parameter (n)	Freq. (f,)	Loss arameter (n)	Freq.	Loss Parameter	Freq (f,)	Loss Parameter (n)
-	816.	0.002	825.	0.022	860.	0.077	91.	0.094	.96	0.031	97.	0.008
2	1915.	0.001	1926.	0.011	1974.	0.050	206.	0.095	223.	0.058	230.	0.019
м	2144.	0.001	2155.	0.010	2206.	0.047	230.	0.093	250.	0.062	257.	0.021
4	3213.	0.001	3225.	0.007	3282.	0.035	342.	0.078	372.	0.074	387.	0.027
S	3738.	0.001	3751.	0.007	3814.	0.034	397	0.073	432.	0.082	452.	0.037
9	4337.	0,001	4351.	900.0	4420.	0.032	459.	0.068	500.	0.087	526.	0.042
7	4987.	0.001	5001.	0.005	5070.	0.027	533.	0.059	577.	0.085	.909	0.040
œ			5372.	0.005	5444.	0.026	571.	0.057	618.	0.087	652.	0.043
6					7081.	0.021	677.	0.049	731.	0.091	777.	090.0
10							759.	0.045	814.	0.087	861.	0.049

TABLE 7 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, unrestrained rotation, unrestrained shear) 11 11 11 Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (Y)

						SHEAR F	SHEAR PARAMETER (9)	R (g)				
		0.1	Steel 1	Steel Face Sheets 1.0		0.9	30		Aluminum Face 200.	m Face Sheets 200.	ts 1000	00.
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter F (n)	req. f.)	Loss rameter (n)	Freq. (f,)	Loss Parameter (n)	Freq.	Loss rameter (n)	Freq (f,)	Loss Parameter (n)
_	465.	0.007	479.	090.0	534.	0.190	102.	0.194	114.	0.056	117.	0.014
2	1098.	0.003	1112.	0.029	1183.	0.123	220.	0.218	261.	0.110	275.	0.032
ю	1229.	0.003	1245.	0.026	1318.	0.116	244.	0.217	291.	0.119	308.	0.035
4	1856.	0.002	1872.	0.018	1950.	0.085	353.	0.192	426.	0.147	458.	0.048
ഗ	2166.	0.002	2182.	0.016	2263.	0.078	405.	0.183	491.	0.165	535.	0.061
9	2519.	0.002	2536.	0.014	2622.	0.071	466.	0.172	565.	0.177	622.	0.069
7	2917.	0.001	2934.	0.012	3018.	090.0	535.	0.153	643.	0.177	709.	0.071
∞	3135.	0.001	3153.	0.011	3241.	0.0576	572.	0.149	688.	0.183	763.	0.076
6	3739.	0.001	3758.	0.010	3854.	0.0515	671.	0.130	806.	0.193	908.	0.098
01			7167.	0.009	4258.	0.0455	748.	0.120	884.	0.191	993.	060.0

TABLE 8
MODAL FREQUENCIES AND MODAL LOSS FACTORS

PTU (zero translation, unrestrained rotation, unrestrained shear) 1.1 3.5 11 11 11 Boundary Condition Aspect Ratio (∆xy) Geometric Parameter (

						SHEAR F	SHEAR PARAMETER (g)	(6)				
		0.1	Steel F	Face Sheets		10.	40		uminum F	Aluminum Face Sheets	ts 1000	UU
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter F (n)	req.	Loss Trameter (n)	Freq.	Loss arameter (n)	Freq.	Loss Parameter (n)	Freq (f,)	Loss Parameter (n)
_	526.	0.016	561.	0.127	753.	0.346	167.	0.242		0.051	194.	0.017
7	1238.	0.007	1276.	0.062	1547.	0.293	347.	0.331	434.	0.108	452.	0.038
က	1387.	0.006	1425.	0.056	1709.	0.283	382.	0.339	485.	0.119	507.	0.042
4	2091.	0.004	2130.	0.038	2443.	0.227	535.	0.336	705.	0.156	751.	0.059
Ŋ	2439.	0.004	2478.	0.033	2805.	0.212	.909	0.337	813.	0.1812	874.	0.071
9	2835.	0.003	2876.	0:030	3222.	0.196	689.	0.330	936.	0.200	1015.	0.082
7	3281.	0.003	3321.	0.025	3663.	0.170	776.	0.307	1053.	0.208	1149.	0.088
∞	3526.	0.002	3567	0.023	3921.	0.164	826.	0.303	1127.	0.219	1237.	0.095
6	4203.	0.002	4246.	0.020	4626.	0.145	956.	0.278	1320.	0.241	1468.	0.114
01	4661.	0.002	4701.	0.018	5061.	1.132	1048.	0.266	1424.	0.249	1589.	0.117

TABLE 9

(zero translation, unrestrained rotation, unrestrained shear) MODAL FREQUENCIES AND MODAL LOSS FACTORS 11 II II Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (Y)

	·		-			SHEAR F	SHEAR PARAMETER (g)	(6)				
		0.1	Steel Fa	Face Sheets 1.0		6.0	30.		uminum 200	Aluminum Face Sheets 200.	ts 1000.	0.
MODE (r)	Freq. (f,)	Loss Parameter (n)	Freq.	Loss Parameter F (n)	req. f.)	Loss Parameter $\frac{n}{n}$	Freq.	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss Parameter (n)
_	352.	0.021	383.	0.165	492.	0.378	150.	0.300		0.080	129.	
5	829.	0.010	865.	0.088	1020.	0.296	305.	0.382	398.	0.158	300.	0.045
က	929.	0.009	996	0.081	1128.	0.284	334.	0.387	443.	0.172	336.	0.050
4	1405.	9000	1445.	0.059	1630.	0.230	465.	0.374	638.	0.217	498.	0.069
ഹ	1639.	0.006	1681.	0.053	1880.	0.217	526.	0370.	731.	0.246	579.	0.081
9	1907.	0.005	1952.	0.049	2166.	0.204	598.	0.359	837.	0.267	672.	0.092
7	2214.	0.005	2259.	0.042	2477.	0.181	672.	0.332	939.	0.275	762.	0.100
<b>∞</b>	2380.	0.004	2427.	0.041	2655.	0.177	715.	0.326	1002.	0.285	820.	0.106
თ	2837.	0.004	2889.	0.036	3143.	0.163	827.	0.298	1167.	0.305	972.	0.127
01	3165.	0.003	3215.	0.033	3462.	0.149	907.	0.284	1256.	0.313	1053.	0.130

TABLE 10

PTU (zero translation, unrestrained rotation, unrestrained shear) 2.0 0.5 MODAL FREQUENCIES AND MODAL LOSS FACTORS | | | | | <del>|</del> | Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (

						SHEAR P	SHEAR PARAMETER (g)	(6)				
	0	1	Steel Fac	Steel Face Sheets 1.0	6.0	0	30.	A)	uminum F	Aluminum Face Sheets 200.	1000	
MODE (r)	Freq. Pa (f,)	Loss Parameter (n)	Freq. (f,)	Loss Parameter (n)	Freq. (f,)	Loss rameter (n)	Freq. (f,)	Loss Parameter (n)	Freq. (f,)	Loss arameter (n)	Freq. (f,)	Loss Parameter (n)
	1825.	0.001	1836.	0.012	1883.	0.052	197.	0.098	214.	0.057	220.	0.018
2	2873.	0.001	2884.	0.008	2939.	0.039	309.	0.085	337.	0.073	349.	0.025
ო	4618.	0.001	4632.	0.006	4700.	0:030	496.	0.067	539.	0.087	566.	0.037
4	6085.	0.001	6102.	0.005	6181.	0.026	649.	0.055	703.	960.0	747.	0.053
ഹ	. 6977	0.001	.9669	0.005	7080.	0.024	758.	0.048	816.	0.094	868.	0.053
ဖ	7062.	0.001	7082.	0.005	7168.	0.023	762.	0.048	821.	0.093	873.	0.054
7			8664.	0.004	8755.	0.020	947.	0.039	1012.	060.0	1079.	0.057
ω					10284.	0.020	1114.	0.034	1186.	0.090	1272.	0.070
6					10890.	0.017	1210.	0.031	1281.	0.085	1366.	0.063
10							1449.	0.027	1531.	0.087	1654.	0.091

TABLE 11 MODAL FREQUENCIES AND MODAL LOSS FACTORS

PTU (zero translation, unrestrained rotation, unrestrained shear)  $2.0\,$  1.5 n n n Boundary Condition Aspect Ratio (∆xy) Geometric Parameter (

TABLE 12 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, unrestrained rotation, unrestrained shear) PTU 2.0 3.5 H H H Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (Y)

	' ]		1	2+0043 00 00 00 00 00 00 00 00 00 00 00 00 00		SHEAR P	SHEAR PARAMETER (g)			 		
).1	.	Stee		race sneets 1.0		0.9		30.	uminum 20	Aluminum Face Sheets 200.	ts 1000	0.0
Freq. Parameter Freq. $(f_r)$ $(f_r)$	Loss arameter Fre (n)	Fre (f		Loss Para <u>m</u> eter (n)	Freg. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Para <u>m</u> eter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter (n)
1182. 0.007 12		15	1219.	0.066	1389.	0.245	320.	0.350	408.	0.145	436.	0.036
1875. 0.005 19		13	1914.	0.043	2100.	0.186	469.	0.344	622.	0.197	683.	0.055
3041. 0.003 30		30	3082.	0.028	3289.	0.134	703.	0.304	955.	0.255	1089.	0.083
4013. 0.003 40		40	4057.	0.023	4284.	0.112	891.	0.274	1215.	0.294	1427.	0.109
4663. 0.002 470		47(	4706.	0.019	4932.	0.098	1019.	0.25	1374.	0.303	1632.	0.120
4696. 0.002 47		47	4740.	0.019	4971.	0.097	1025.	0.248	1387.	0.300	1649.	0.121
5808. 0.002 58	<del> </del>	286	5854.	0.015	6090.	0.080	1242.	0.213	1653.	0.308	1996.	0.137
6846. 0.001 68		89	6894.	0.014	7151.	0.072	1439.	0.190	1903.	0.316	2343.	0.158
7342. 0.001 73		73	7387.	0.012	7630.	0.065	1546.	0.177	2008.	0.313	2465.	0.159
88	88	89	8948.	0.012	9235.	0.061	1831.	0.156	2367.	0.328	2987.	0.197

TABLE 13

(zero translation, unrestrained rotation, unrestrained shear) MODAL FREQUENCIES AND MODAL LOSS FACTORS PTU 2.0 4.5 II II II Boundary Condition Aspect Ratio (Δxy) Geometric Parameter

,						SHEAR F	SHEAR PARAMETER (g)	R (g)				
		0.1	Steel F	Face Sheets 1.0		6.0	(n)	30.	Aluminum Face	m Face Sheets		000
MODE (r)	Freq. (f <sub>r</sub> )	Loss Para <u>me</u> ter (n)	Freq. (f <sub>r</sub> )	Loss Parameter $(\overline{n})$	Freq. (f,)	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss rameter (n)	Freq (f,)	Loss Parameter (n)
	792.	0.011	828.	0.093	982.	0.306	295.	0.386		0.155	289.	
2	1261.	0.007	1301.	0.066	1482.	0.250	429.	0.389	584.	0.210	453.	0.064
က	2052.	0.005	2098.	0.047	2319.	0.198	638.	0.354	894.	0.271	722.	0.094
4	2712.	0.004	2765.	0.040	3020.	0.178	803.	0.326	1136.	0.313	944.	0.120
S.	3165.	0.004	3220.	0.036	3486.	0.162	915.	0.300	1283.	0.324	1079.	0.132
9	3183.	0.004	3238.	0.036	3509.	0.160	920.	0.299	1296.	0.322	1091.	0.134
7	3958.	0.003	4017.	0.030	4309.	0.141	1110.	0.262	1541.	0.333	1320.	0.151
ω	4670.	0.003	4736.	0.029	5069.	0.134	1283.	0.239	1774.	0.345	1547.	0.172
σ,	5038.	0.003	5102.	0.026	5424.	0.124	1375.	0.224	1868.	0.343	1631.	0.175
10	.9609	0.003	6181.	0.025	6584.	0.024	1626.	0.203	2206.	0.362	1968.	0.212

TABLE 14

PTU (zero translation, unrestrained rotation, unrestrained shear) 4.0 0.5 MODAL FREQUENCIES AND MODAL LOSS FACTORS (X) Boundary Condition Aspect Ratio (∆xy) Geometric Parameter (

						SHEAR F	SHEAR PARAMETER (g)	(a)				
	0.		Steel Face Sh 1.0	ace Sheets	6.0		30.	A1	uminum Fa	Aluminum Face Sheets	1000	
MODE (r)	Freq. F (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss arameter (n)	Freq.	Loss Parameter (n)
_	5968.	0.010	5986.	9000	6068.	0.028	642.	0.059		0.097	739.	0.048
2	6885.	0.001	6904.	0.005	.0669	0.025	750.	0.052	811.	0.097	863.	0.051
က	8436.	0.001	8459.	0.004	8554.	0.022	933.	0.044	1002.	960.0	1069.	0.057
4	10513.	0.002	10545.	0.004	10653.	0.020	1188.	0.036	1266.	0.093	1355.	0.064
2	11583.	0.002	11612.	0.004	13508.	0.017	1525.	0.028	1611.	0.087	1730.	0.073
Q			13377.	0.003	16841.	0.016	1942.	0.022	2036.	0.079	2186.	0.080
7			13587.	0.003	20602.	0.015	2446.	0.018	2545.	0.070	2732.	0.087
œ					20684.	0.014	2481.	0.018	2585.	0.073	2792.	0.101
თ							2596.	0.017	2699.	0.070	2910.	0.098
10		·					2784.	90.0	2888.	0.067	3105.	960.0

TABLE 15 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, unrestrained rotation, unrestrained shear) Boundary Condition = PTU Aspect Ratio  $(\Delta xy)$  = 4.0 Geometric Parameter (Y) = 1.5

	,					SHEAR	SHEAR PARAMETER (g)	(6)	i			
	0.	0.12	Steel Face Sh 1.2	ce Sheets 2	6.0		30.	Al	Aluminum Face 200.	ace Sheets	1000	
MODE (r)	Freg. P (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq. P. (f,)	Loss rameter (n)	Freq.	Loss Parameter $(\overline{n})$	Freq.	Loss rameter (n)	Freq.	Loss Parameter (n)
_	3514.	0.002	3537.	0.014	3631.	090.0	641.	0.153	I ~	0.203	873.	0.085
2	4090.	0.002	4114.	0.012	4211.	0.053	744.	0.138	.968	0.208	1014.	0.093
က	5060.	0.002	5087.	0.010	5190.	0.045	916.	0.118	1090.	0.212	1246.	0.106
4	6399.	0.002	6430.	0.009	6541.	0.039	1157.	0.098	1355.	0.212	1564.	. 0.123
r.	8182.	0.003	8220.	0.007	8345.	0.033	1473.	0.079	1697.	0.204	1976.	0.142
9	10330.	0.003	10381.	900.0	10522.	0.028	1864.	0.064	2110.	0.191	2470.	0.159
7			12504.	0.005	13105.	0.025	2337.	0.051	2600.	0.175	3050.	0.175
œ	daya ir		12943.	900.0	13444.	0.027	2370.	0.051	2644.	0.081	3128.	0.192
თ			13260.	900.0	14004.	0.026	2477.	0.049	2752.	0.174	3249.	0.189
10					14835.	0.024	2654.	0.045	2930.	0.168	3445.	0.190

TABLE 16 MODAL FREQUENCIES AND MODAL LOSS FACTORS

PTU (zero translation, unrestrained rotation, unrestrained shear) 4.0 3.5 11 11 11 Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (

	1000.	Loss Parameter (n)	. 0.107	. 0.120	. 0.140	0.165	0.191	. 0.217	. 0.241	. 0.257	. 0.255	0.261
		Freq (f,)	1427.	1648.	2012.	2503.	3140.	3887.	4747.	4885.	5054.	5328.
	Face Sheets J.	Loss Parameter (n)	0.295	0.307	0.322	0.333	0.332	0.321	0.304	0.313	0.303	0.295
	Aluminum Face 200.	Freq.	1217.	1387.	1663.	2030.	2498.	3049.	3690.	3760.	3897.	4119.
(g)		Loss Parameter (n)	0.286	0.263	0.232	0.1978	0.164	0.134	0.108	0.110	0.104	0.100
SHEAR PARAMETER (g)	30	Freq.	888.	1018.	1235.	1536.	1932.	2418.	3004.	3049.	3182.	3398.
SHEAR P		Loss Parameter (n)	0.119	0.105	0.089	0.075	0.062	0.052	0.044			
	6.0	Freq. (f,)	4228.	4877.	5976.	7489.	9516.	11952.	14835.			
	ce Sheets 0	Loss Parameter (n)	0.024	0.021	0.017	0.014	0.012	0.010	0.008	0.008	0.008	0.008
	Steel Face 1.0	Freq. (f <sub>r</sub> )	3996.	4641.	5730.	7229.	9228.	11636.	14484.	14832.	15468.	16364.
	-1	Loss Parameter $\frac{n}{n}$	0.003	0.002	0.002	0.002	0.001	0.001	0.001	0.001	0.001	0.001
	0.	Freq. Pa (f <sub>r</sub> )	3950.	4595.	5626.	5682.	7180.	9174.	11575.	14420.	14761.	15391.
		MODE (r)	_	2	ო	4	ဌ	9	^	8	6	20

TABLE 17

MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, unrestrained rotation, unrestrained shear) 11 Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (

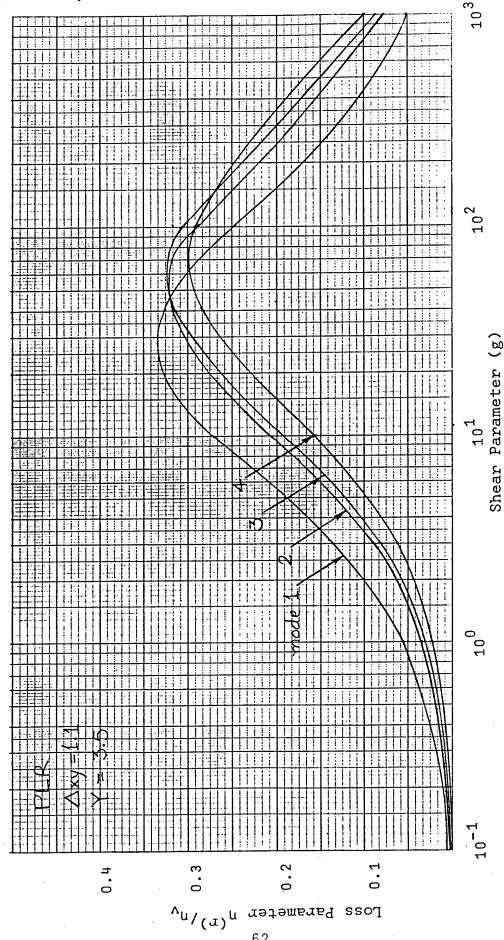
	Sheets 1000.0	Loss Parameter Freq. Parameter (n) (f <sub>r</sub> ) (f <sub>r</sub> )	0.314 943. 0.118	0.328 1090. 0.131	0.346 1330. 0.152	0.361 1654. 0.179	0.365 2073. 0.207	0.360 2563. 0.233	0.349 3127. 0.260	0.360 3210. 0.274	0.352 3322. 0.273	
	Aluminum Face 200.0	Freq. (f <sub>r</sub> )	1138. 0	1296. 0	1551.	1890.	2324.	2833. (	3426.	3499.	3624. 0	2995. 0.137 3827. 0.345
(6)		Loss Parameter (n)	0.338	0.315	0.283	0.247	0.210	0.178	0.149	0.152	0.149	
SHEAR PARAMETER (9)	30.0	Freq. (f <sub>r</sub> )	801.	915.	1105.	1369.	1714.	2138.	2650.	2691.	2806.	
SHEAR P		Loss Parameter (n)	0.185	0.171	0.153	0.138	0.124	0.112	0.102	0.105	0.100	
	6.0	Freq. (f,)	2987.	3457.	4249.	5351.	6816.	8605.	10745.	11481.	12224.	
	Face Sheets 1.0	Loss Para <u>m</u> eter (n)	0.042	0.038	0.033	0.029	0.026	0.023	0.021	0.022		
	Steel Fa	Freq. (f <sub>r</sub> )	2727.	3180.	3943.	5006.	6414.	8133.	10191.	10426.		
		Loss Para <u>m</u> eter (n)	0.005	0.004	0.004	0.003	0.003	0.002	0.002	0.002		
	0	Freq. Pā (f <sub>r</sub> )	2673.	3124.	3882.	4936.	6333.	8040.	10082.	10306.		
		MODE (r)	_	2	m	4	ហ	ဖ	7	ω	6	

## 3.2.2 PLR Boundary Conditions

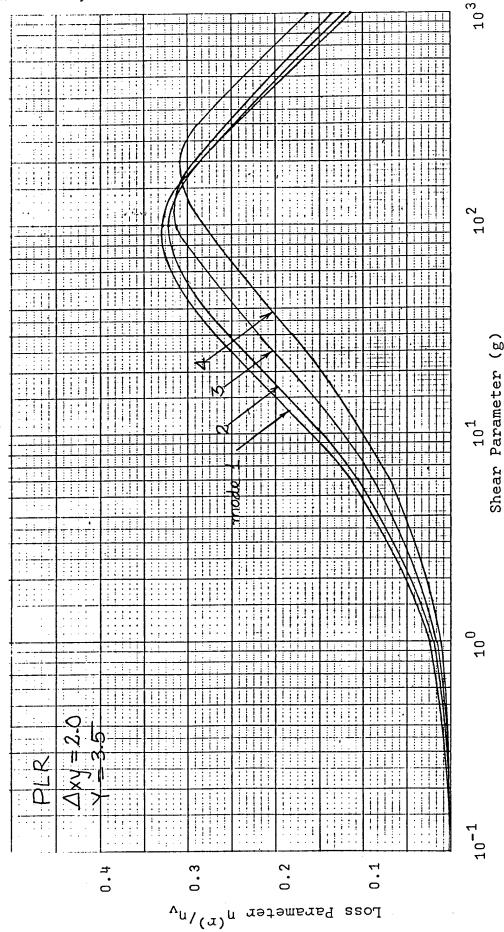
Damping properties of sandwich plates with PLR (fixed) boundary conditions are given in Figures 27 through 29. Only one value of the geometry parameter, Y = 3.5, is considered. This corresponds to the situation of equal face sheet thicknesses.

Natural frequencies of sandwich plates with PLR boundary conditions are given in Figures 30 through 32. Reference frequencies are given in Table 5.

A tabular presentation of the data for damping and natural frequencies of plates with PLR boundary conditions is given in Tables 18 through 20.



a sandwich rectangular plate by conditions,  $\Delta xy = 1.1$ , Y = Damping of a PLR boundary Figure 27



>

2.0,

11

conditions, Axy

Damping of a PLR boundary

28

sandwich rectangular plate

63

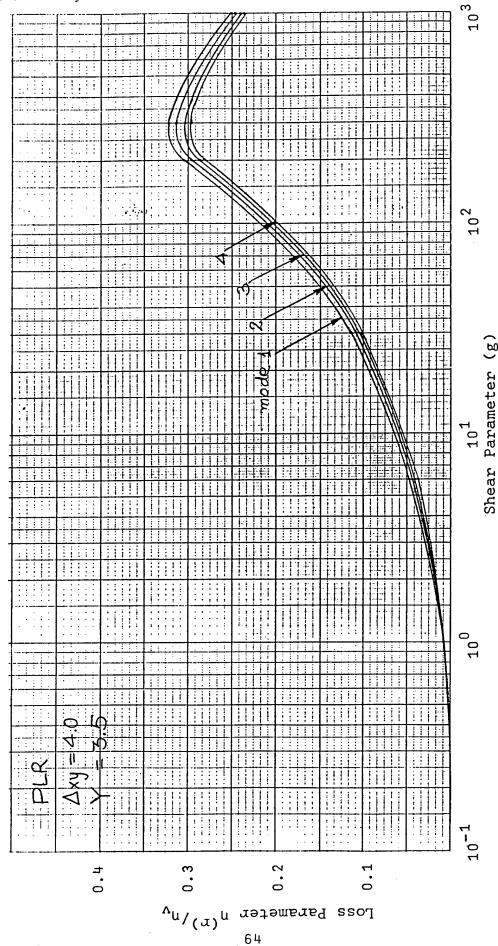
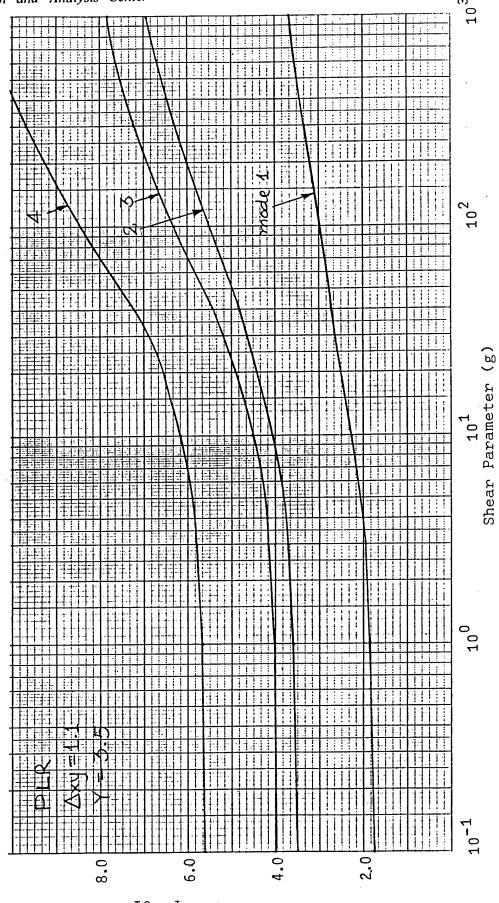


Figure 29 Damping of a sandwich rectangular plate, PLR boundary conditions,  $\Delta xy = 4.0$ , Y = 3.5



sandwich = 1.1, Y =

rectangular

of a

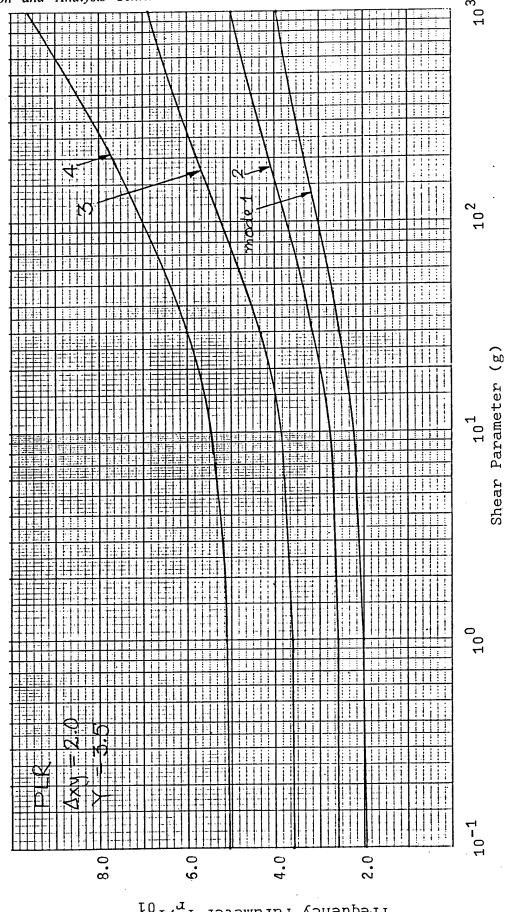
Natural frequencies plate, PLR boundary

30

 $\Delta xy$ 

conditions,

Frequency Parameter f<sub>r</sub>/f<sub>01</sub>



sandwich

rectangular

of a

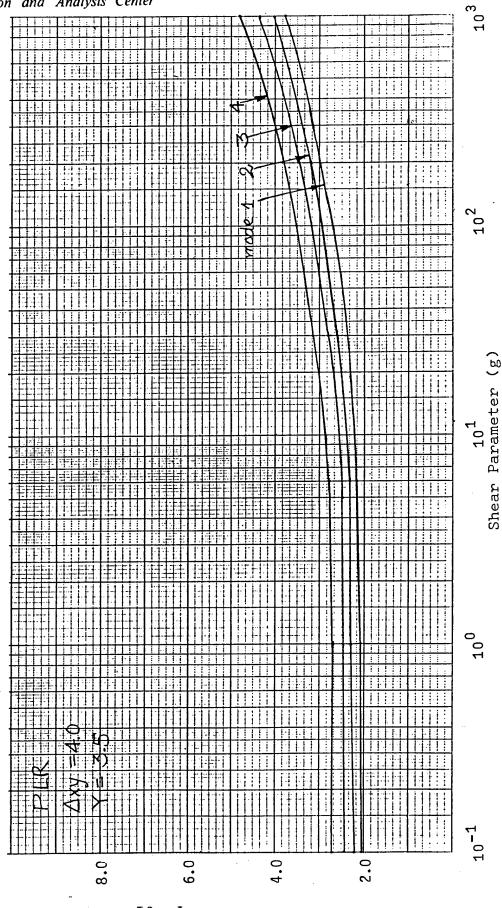
Natural frequencies plate, PLR boundary

31

11

conditions,

Leedneuch barameter f<sup>r</sup>/f<sup>01</sup>



sandwich

rectangular

of a

frequencies PLR boundary

Natural plate, P

32

п

 $\Delta xy$ 

conditions,

Frequency Parameter fr/f<sub>01</sub>

TABLE 18 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, zero rotation, zero shear) Boundary Condition Aspect Ratio (∆xy) Geometric Parameter

						SHEAR PARAMETER (g)	ARAMET	ER (g)				
		0.1	Steel F	Steel Face Sheets 1.0		10.	4	40.	Aluminum Face 100.	m Face Sheets 100.	1000	00
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freg. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss arameter (n)	Freq (f,)	Loss Parameter (n)
-	959.	0.007	985.	0.055	1171.	0.272	259.	0.325	296.	0.248	346.	0.051
2	1852.	0.004	1884.	0.034	2130.	0.205	457.	0.316	529.	0.291	651.	0.077
က	2092.	0.003	2123.	0.03	2379.	0.192	504.	0.313	587.	0.306	735.	980.0
4	2947.	0.002	2982.	0.023	3278.	0.155	677.	0.283	792.	0.293	1004.	00.100
Ŋ	3312.	0.003	3348.	0.021	3643.	0.148	752.	0.276	868.	0.311	1126.	0.110
9	3892.	0.002	3928.	0.018	4244.	0.135	860.	0.263	. 766	0.314	1315.	0.126
7	4362.	0.002	4398.	0.018	4718.	0.118	953.	0.243	1099.	0.289	1438.	0.125
∞	4710.	0.002	4748.	0.015	5083.	0.111	1017.	0.236	1178.	0.287	1548.	0.133
6	5397.	0.002	5437.	0.013	5778.	0.105	1164.	0.209	1313.	0.287	1784.	0.147
10	. 2009	0.001	6045.	0.012	6382.	0.092	1269.	0.205	1451.	0.271	1908.	0.152

TABLE 19
MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, zero rotation, zero shear) PLR 2.0 3.5 11 11 11 Boundary Condition Aspect Ratio (∆xy) Geometric Parameter

						SHEAR P	SHEAR PARAMETER (g)	ER (g)				
		0.1	tee	Face Sheets   1.0		6.0		30.	Aluminum Face	Face Sheets		1000
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter F (n)	req.	Loss Parameter (n)	Freq.	oss ameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq (f,)	Loss Parameter (n)
_	2369.	0.002	2395.	0.024	2529.	0.116	525.	0.269	706.	0.285	827.	0.114
2	3049.	0.002	3080.	0.022	3240.	0.107	.029	0.252	892.	0.284	1050.	0.121
'n	4275.	0.002	4311.	0.018	4499.	0.089	919.	0.221	1213.	0.292	1451.	0.134
4	.6609	0.001	6140.	0.0132	6360.	0.069	1277.	0.181	1639.	0.310	2037.	0.163
ഹ	6200.	0.001	6238.	0.012	6439.	0.065	1284.	0.159	1661.	0.302	2042.	0.188
9	6786.	0.001	6824.	0.012	7029.	0.061	1411.	0.152	1783.	0.303	2210.	0.189
7	7975.	0.001	8017.	0.010	8239.	0.053	1651.	0.136	2065.	0.292	2562.	0.187
<b>ω</b>	8503.	0.001	8550.	0.010	.0088	0.054	1749.	0.138	2213.	0.280	2768.	0.188
6	9469.	0.001	9511.	600.0	9741.	0.046	1972.	0.119	2419.	0.275	3000.	0.197
10	11406.	0.001	11457.	0.008	11734.	0.042	2334.	0.104	2852.	0.252	3603.	0.214

TABLE 20 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, zero rotation, zero shear) PLR 4.0 3.5 Boundary Condition Aspect Ratio (Δxy) Geometric Parameter

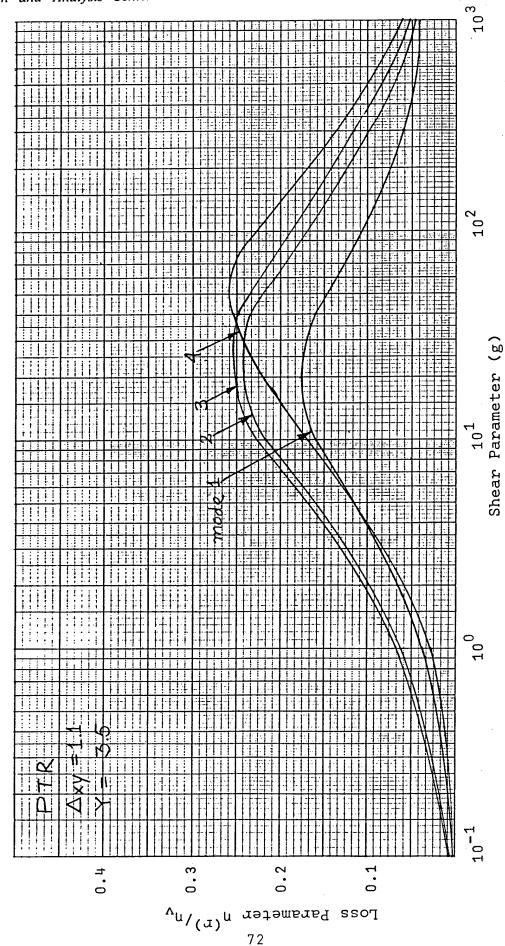
						SHEAR F	SHEAR PARAMETER (g)	ER (g)				
		0.1	Steel F	Face Sheets 1.0		0.9		30.	uminum 20	Aluminum Face Sheets 200.	1000	00
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter Freg. (n)	Freq. (f,)	Loss Parameter (n)	Freq. (f,)	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq (f,)	Loss Parameter (n)
_	8411.	0.001	8447.	600.0	8641.	0.048	1733.	0.110	2120.	0.306	2721.	0.251
2	8859.	0.001	8898.	600.0	.9104.	0.048	1836.	0.110	2239.	0.298	2858.	0.247
က	9693.	0.001	9735.	0.009	. 1966	0.046	2020.	0.108	2454.	0.289	3114.	0.241
4	10837.	0.001	10882.	0.008	11126.	0.045	2285.	0.105	2759.	0.284	3480.	0.241
ഹ	12635.	0.001	12686.	0.008	12961.	0.041	2664.	0.097	3195.	0.275	4033.	0.240
9	14903.	0.001	14960.	0.007	15268.	0.037	3148.	980.0	3734.	0.263	4723.	0.244
7	17806.	0.001	17870.	900.0	18215.	0.032	3759.	0.074	4398.	0.245	5574.	0.251
<b>&amp;</b>	20458.	0.001	20524.	0.005	20886.	0.031	4427.	0.064	5083.	0.233	.9689	0.264
6	22487.	0.000	22554.	0.004	22920.	0.025	4961.	0.045	5524.	0.181	6854.	0.266
10	23007.	0.000	23074.	0.004	23433.	0.023	5094.	0.043	5663.	0.174	.6669	0.259

## 3.2.3 PTR Boundary Conditions

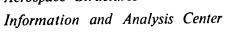
Damping properties of sandwich plates with PTR (simply-supported, riveted) boundary conditions are given in Figures 33 through 35. Again, only a single value of the geometry parameter, Y = 3.5, is considered.

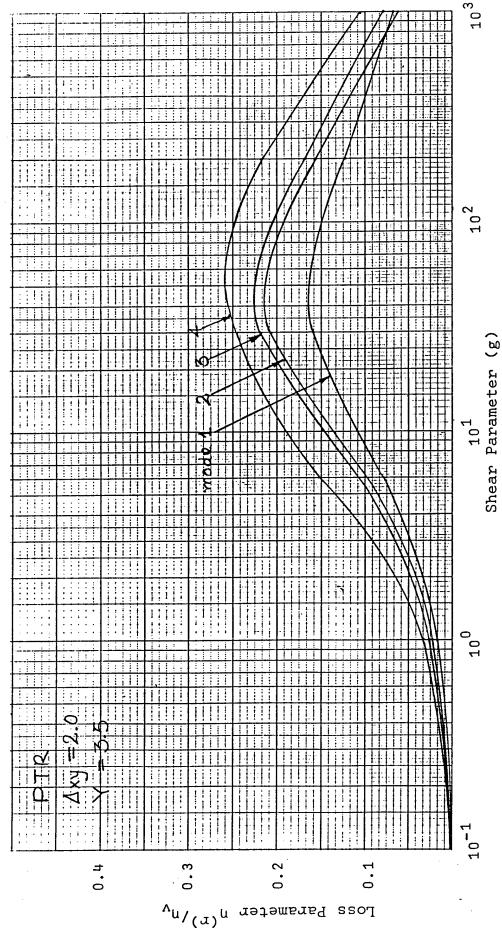
Natural frequencies of sandwich plates with PTR boundary conditions are given in Figures 36 through 38. Reference frequencies are given in Table 5.

A tabular presentation of the data for damping and natural frequencies of plates with PTR boundary conditions is given in Tables 21 through 23.

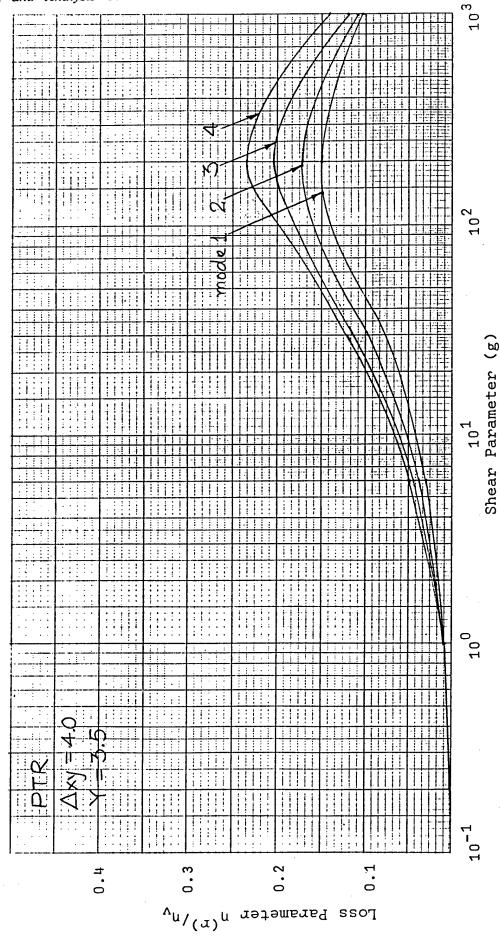


PTR plate, = 3.5 Damping of a sandwich rectangular boundary conditions,  $\Delta xy = 1.1$ , Y 33

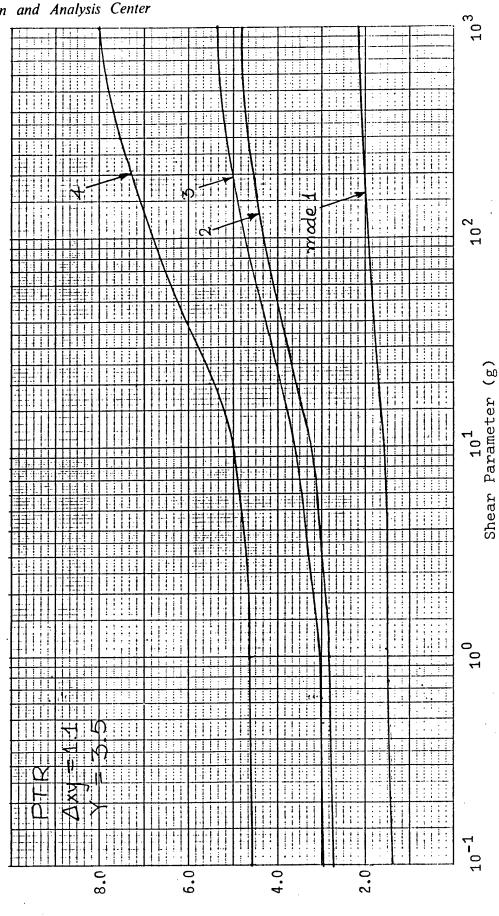




plate, = 3.5 sandwich rectangular litions,  $\Delta xy = 2.0$ , Y Damping of a sandwich rec boundary conditions, Axy 34



plate, = 3.5 a sandwich rectangular anditions,  $\Delta xy = 4.0$ , Y Damping of a sandwich red boundary conditions,  $\Delta xy$ 35



sandwich = 1.1, Y =

11

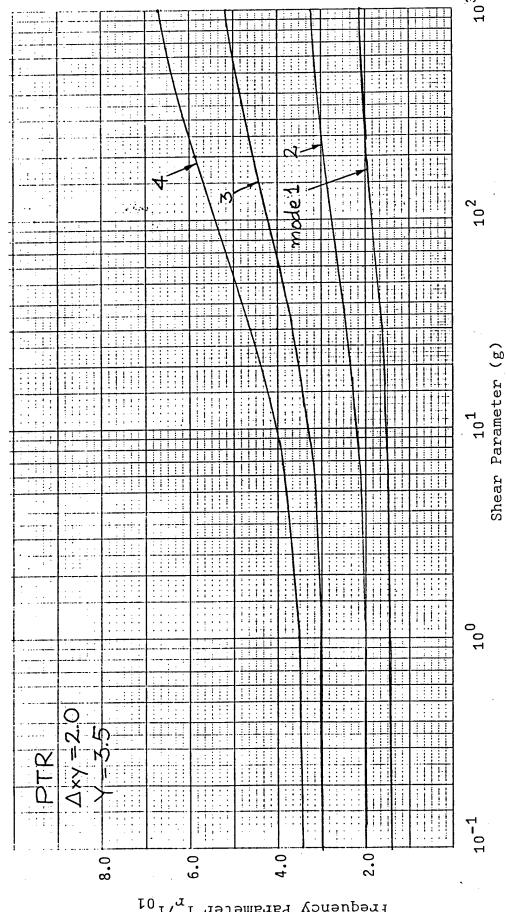
of a rectangular conditions,  $\Delta xy =$ 

Natural frequencies plate, PTR boundary

36

Figure

Frequency Parameter  $f_{\rm r}/f_{\rm 01}$ 



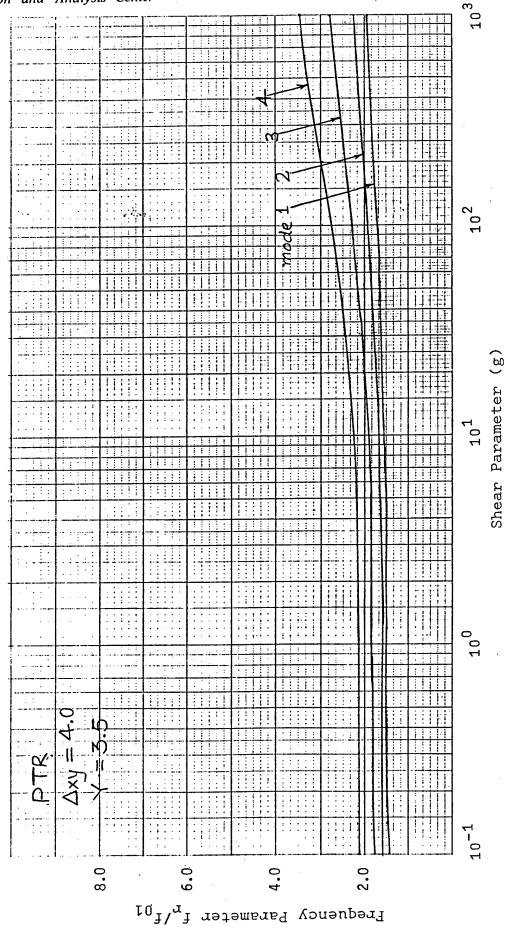
sandwich = 2.0, Y

rectangular

Natural frequencies of a rectangular plate, PTR boundary conditions, Axy

37

Frequency Parameter  $f_{\nu}/f_{01}$ 



.5 rectangular sandwich tions,  $\Delta xy = 4.0$ , Y conditions, Axy . رر ψO Natural frequencies plate, PTR boundary 38

TABLE 21 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, unrestrained rotation, zero shear) PTR 1.1 3.5 11 II II Boundary Condition Aspect Ratio (Δxy) Geometric Parameter

						SHEAR P	SHEAR PARAMETER (g)	R (g)				
	0	0.1	Steel Fa	Face Sheets 1.0		10.	4	40.	uminum F	Aluminum Face Sheets 300.	1000	00.
MODE (r)	Freq. (fr)	Loss Parameter $(\frac{1}{n})$	Freq. (f <sub>r</sub> )	.oss .ameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss ara <u>me</u> ter (n)	Freq (f,)	Loss Parameter (n)
-	751.	0.004	764.	0.037	854.	0.161	172.	~0.161	192.	90.0	197.	0.043
2	1443.	0.007	1486.	0.059	1743.	0.217	367.	0.236	436.	0.093	454.	0.039
က	1548.	0.007	1579.	0.062	1890.	0.227	400.	0.249	485.	0.111	510.	0.056
4	2409.	0.003	2444.	0.030	2719.	0.171	568.	0.249	709,	0.133	752.	090.0
2	2719.	0.003	2762.	0.032	3091.	0.181	648.	0.266	821.	0.150	876.	0.063
9	3113.	0.003	3154.	0.027	3492.	0.170	729.	0.268	940.	0.172	1016.	0.083
7	3669.	0.002	3707.	0.021	4024.	0.137	830.	0.240	1065.	0.175	1151.	0.080
ω	3888.	0.002	3928.	0.020	4265.	0.137	880.	0.242	1137.	0.185	1238.	0.090
6	4372.	0.004	4449.	0.032	4969.	0.152	1026.	0.237	1344.	0.196	1475.	960.0
10	5054.	0.003	5139.	0.029	5474.	0.110	1119.	0.216	1443.	0.216	1592.	0.110

TABLE 22

PTR (zero translation, unrestrained rotation, zero shear) 2.0 3.5 MODAL FREQUENCIES AND MODAL LOSS FACTORS 11 11 11 Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (Y)

			1 1			SHEAR P	SHEAR PARAMETER (g)	(6)				
	0.	1	Steel Fa	Face Sheets 1.0		6.	30		Aluminum Face 200.	Face Sheets 0.	1000	.00
	Freq. P (f <sub>r</sub> )	Loss Parameter Freq. (n)		Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter $(\overline{n})$	Freq. (f,)	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	Loss Parameter (n)
<del> </del>	1707.	0.002	1721.	0.018	1789.	0.079	353.	0.160	410.	0.123	441.	0.068
	2374.	0.003	2401.	0.024	2532.	0.106	519.	0.207	629.	0.157	685.	0.064
· · · · · ·	3541.	0.002	3576.	0.021	3752.	0.098	772.	0.218	973.	0.204	1091.	0.078
	4117.	0.004	4198.	0.039	4571.	0.153	984.	0.244	1248.	0.218	1427.	0.105
******	5092.	0.002	5146.	0.021	5415.	0.098	1113.	0.200	1416.	0.233	1633.	0.114
	5162.	0.002	5210.	0.018	5449.	0.086	1126.	0.204	1425.	0.248	1655.	0.108
	6464.	0.001	6209.	0.013	6738.	0.067	1370.	0.167	1709.	0.239	2000.	0.122
	7323.	0.001	7376.	0.014	7653.	0.068	1548.	0.168	1966.	0.266	2356.	0.137
	8075.	0.001	8117.	0.010	8341.	0.053	1690.	0.140	2081.	0.254	2475.	0.144
i	9642.	0.002	9692.	0.011	10003.	. 0.057	2015.	0.148	2495.	0.255	3005.	0.164

TABLE 23 MODAL FREQUENCIES AND MODAL LOSS FACTORS

PTR (zero translation, unrestrained rotation, zero shear) 4.0 3.5 # # Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (

						SHEAR F	SHEAR PARAMETER (g)	(g)				
! !		0.1	Steel F	Face Sheets 1.0		6.	30		uminum F	Aluminum Face Sheets	ts 1000	
(r)	Freg. (f,)	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss arameter (n)	Freq.	Loss Parameter	Freq. (f_)	Loss arameter (n)	Freq.	Loss Parameter
<i>-</i> -	5920.	0.001	5935.	0.006	6020.	0.030	1 m	0.072	1279	0 148	1420	
7	6415.	0.001	6440.	0.007	6557.		1272.	0.097	1462.	0.171	1650	0.100
m	7335.	0.001	7364.	0.008	7522.	0.043	1488.	0.115	1756.	0.202	2018	0.110
4	8640.	0.001	8676.	0.009	8871.	0.047	1787.	0.120	2142	0 234		121.0
.c	10500.	0.001	10543.	0.008	10781.	0.044	2184.	0 114	2633		2370.	0.143
9	12783.	0.001	12840.	0.008	1263/				, too	062.0	3163.	0.164
	13740		12000				70/3.	0.103	3206.	0.256	3925.	0.187
. (			13880.	0.160	14582.	080.0	3266.	0.089	3874.	0.250	4804.	0.211
∞	14863.	0.001	14983.	0.130	15612.	990.0	3298.	0.188	4138.	0.250	4999.	0.180
6	15560.	0.001	15620.	0.007	15950.	0.036	3481.	0.155	4284.	0.236	5174.	0.179
10	16520.	0.001	16620.	0.010	17153.	0.053	3762.	0.124	4519.	0.231	5458	188
										1	•	007.0

## 3.2.4 PWU Boundary Conditions

A situation that might lead to PWU or PWR boundary conditions is shown in Figure 39. A structure is fabricated by butt welding of plate sections that contain an integral damping treatment. One leg of the weldment sees a constraint on shearing of the sandwich core (PWR), while the other [PWU] does not. Both see some restraint on bending rotation at the welded boundary but it is not held exactly to zero. The restraint is approximated as a pure stiffness and is evaluated, somewhat arbitrarily, as The degree of elastic restraint is taken to be equal to the rotational stiffness of the hypothetical plate used to calculate the reference frequencies. The hypothetical plate has the dimensions of the actual plate but with a flexural stiffness (EI) eqv equal to the sum of the flexural stiffnesses of the upper and lower face sheets. The rotational stiffness at the edge of the hypothetical plate is calculated for that edge unrestrained and a clamped condition imposed on the opposite edge.

Damping as a function of the shear parameter for the first four modes of a rectangular sandwich plate with PWU boundary conditions and a geometry parameter of Y = 3.5 is shown in Figures 40 through 42.

Natural frequencies for sandwich plates with PWU boundary conditions are given in Figures 43 through 45. Reference frequencies are given in Table 5.

A tabular presentation of the data in Figures 40 through 45 is given in Tables 24 through 26, as well as results for the fifth and higher modes.

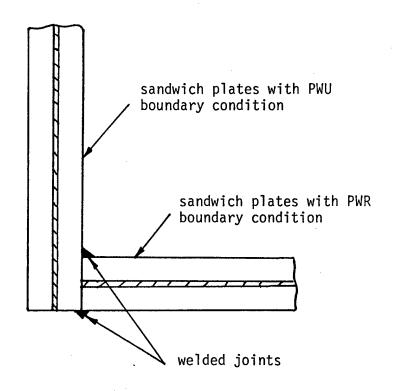
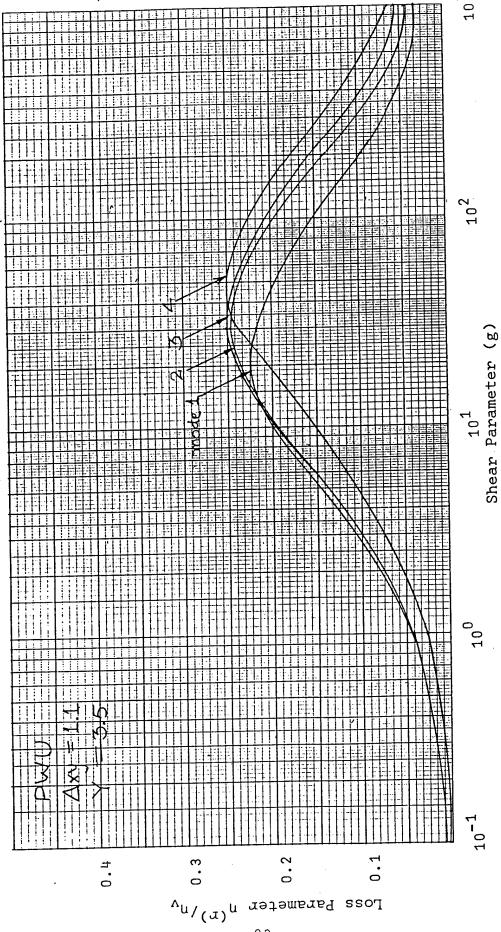


Figure 39 Sandwich plates with PWU and PWR boundary conditions

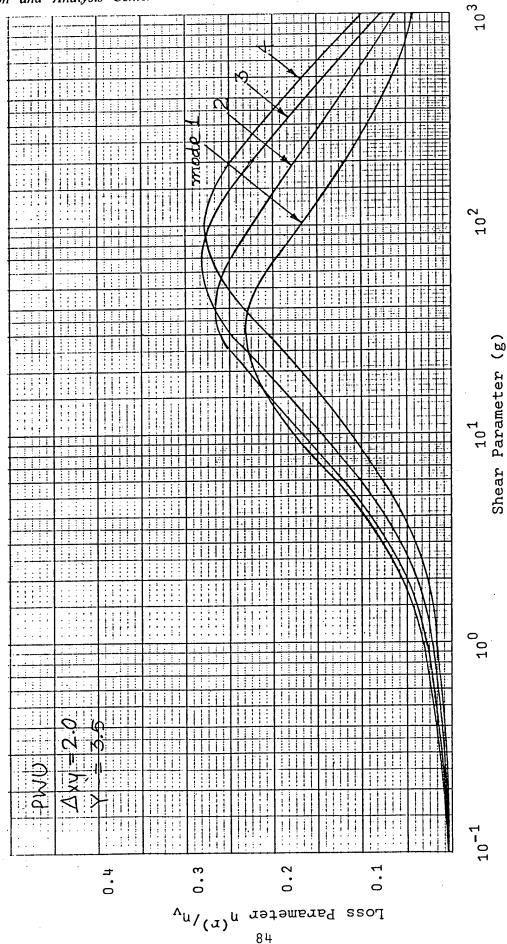


att.

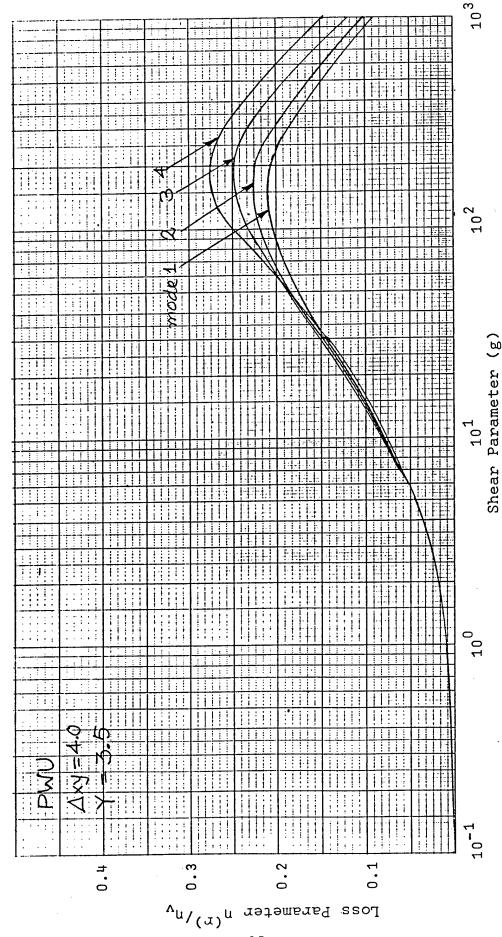
a sandwich rectangular unditions,  $\Delta xy = 1.1$ , Y

Damping of a sandwich rec boundary conditions,  $\Delta xy$ 

0 †



plate, = 3.5 sandwich rectangular 2.0, н conditions, ಗ ψ Damping o boundary 다



PWU

plate, = 3.5

sandwich rectangular itions,  $\Delta xy = 4.0$ , Y

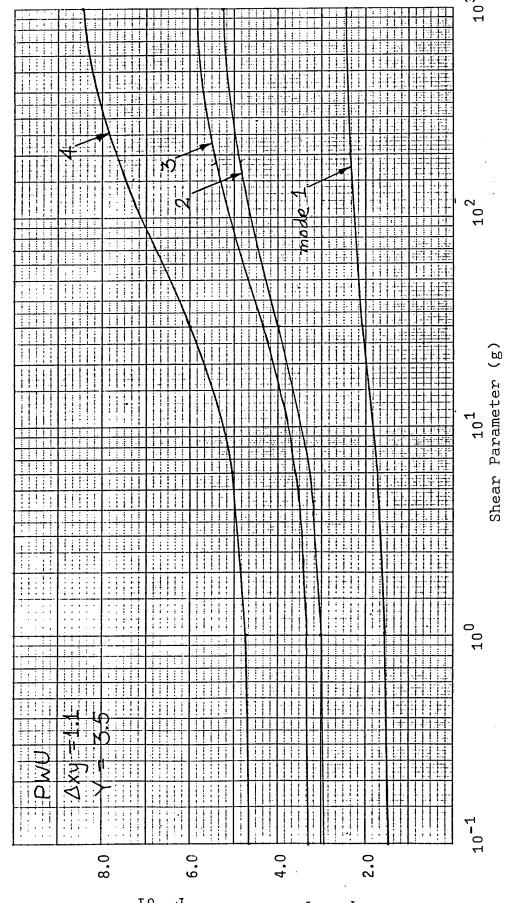
conditions,

ಗ of

Damping o boundary

42

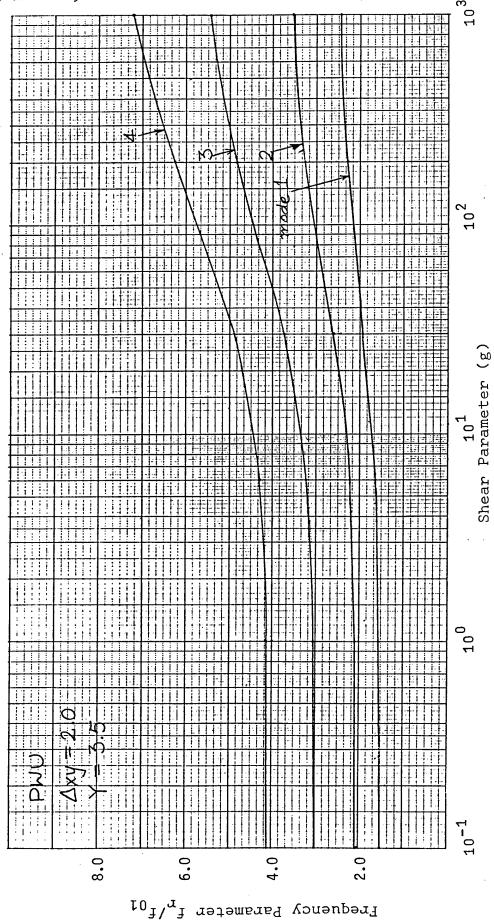
Figure



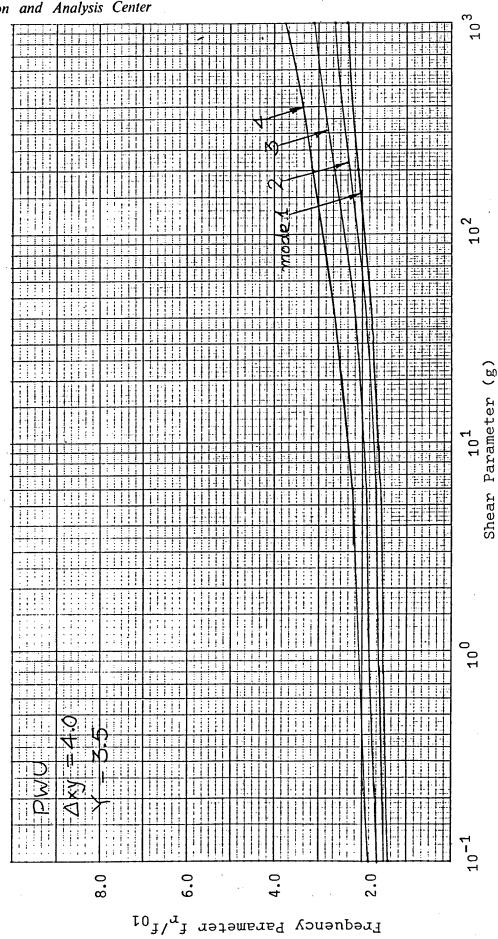
rectangular  $\Delta xy = 1.1, Y$ 

Natural frequencies of a PWU boundary conditions,

Exequency Parameter  $f_{\nu}^{f}/f_{01}$ 



sandwich plate, rectangular  $\Delta xy = 2.0, Y$ Natural frequencies of a PWU boundary conditions, Figure 44



sandwich plate,
= 3.5 rectangular  $\Delta xy = 4.0, 1$ Natural frequencies of a PWU boundary conditions,

TABLE 24

MODAL FREQUENCIES AND MODAL LOSS FACTORS

 $P_{\rm WU}$  (zero translation, elastically restrained rotation, unrestrained shear) 1.1 3.5n n n Boundary Condition Aspect Ratio (∆xy) Geometric Parameter (

						SHEAR F	SHEAR PARAMETER (g)	R (g)				
		0.1	Steel F	Steel Face Sheets 1.		6.	30		luminum Fac	Aluminum Face Sheets		1000
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Para <u>m</u> eter (n)	Freq. (f <sub>r</sub> )	Loss arameter (n)	Freq.	Loss arameter (n)	Freq.	oss ameter (n)	Freq.	Loss Parameter (n)
_	780.	0.008	805.	0.066	907.	0.207	195.	0.215	1 01	0.075	231.	0.026
2	1552.	0.004	1583.	0.041	1728.	0.170	374.	0.280	462.	0.134	492.	0.038
က	1719.	0.004	1751.	0.038	1902.	0.162	409.	0.283	511.	0.145	548.	0.041
4	2452.	0.003	2486.	0.028	2654.	0.130	562.	0.274	723.	0.184	794	0.056
S	2815.	0.003	2850.	0.025	3024.	0.121	633.	0.270	824.	0.210	917.	0.067
9	3238.	0.002	3274	0.023	3458.	0.111	718.	0.261	941.	0.228	1061.	0.076
7	3679.	0.002	3715.	0.020	3899.	0.097	805.	0.242	1052.	0.235	1195.	0.083
ω	3940.	0.002	3977.	0.019	4167.	0.093	856.	0.237	1122.	0.244	1284.	0.089
б	4657.	0.002	4697.	0.016	4900.	0.082	991.	0.211	1304.	0.257	1518.	0.106
10	5077.	0.001	5114.	0.015	5308.	0.076	1080.	0.206	1400.	0.270	1638.	0.112

TABLE 25 MODAL FREQUENCIES AND MODAL LOSS FACTORS

PWU (zero translation, elastically restrained rotation, unrestrained shear)  $2.0\,$  3.5 H H Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (

						SHEAR F	SHEAR PARAMETER (9)	(a)				
		0.1	Steel F	Steel Face Sheets 1.	9		30		uminum Fa	Aluminum Face Sheets	1000	
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq. (f,)	Loss arameter (n)	Freq.	Loss arameter (n)	Freq.	Loss $\frac{\log s}{(n)}$	Freq.	Loss Parameter (n)
_	1801.	0.003	1827.	0.031	1952.	0.132	407.	0.230	487.	0.123	518.	0.042
2	2446.	0.003	2477.	0.027	2631.	0.123	552.	0.257	694.	0.172	755.	0.055
ო	3591.	0.002	3628.	0.021	3809.	0.102	789.	0.246	1024.	0.228	1156.	0.078
4	4904.	0.002	4941.	0.015	5133.	0.078	1036.	0.204	1329.	0.249	1531.	0.099
ഹ	5260.	0.002	5301.	0.015	5513.	0.079	1117.	0.209	1461.	0.273	1716.	0.114
9	5495.	0.002	5532.	0.014	5729.	0.073	1158.	0.326	1483.	0.317	1729.	0.401
7	6604.	0.001	6643.	0.012	6856.	0.063	1379.	0.346	1761.	0.319	2090.	0.401
∞	7447.	0.001	7493.	0.012	7732.	0.061	1543.	0.369	1985.	0.323	2413.	0.402
6	8072.	0.001	8113.	0.010	8336.	0.054	1678.	0.371	2113.	0.323	2554.	0.398
01	10072.	0.001	10084.	0.001	10340.	0.047	2041.	0.362	2531.	0.308	3118.	0.376

TABLE 26 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, elastically restrained rotation, unrestrained shear) PWU 4.0 3.5 11 II II Boundary Condition Aspect Ratio (Δxy) Geometric Parameter

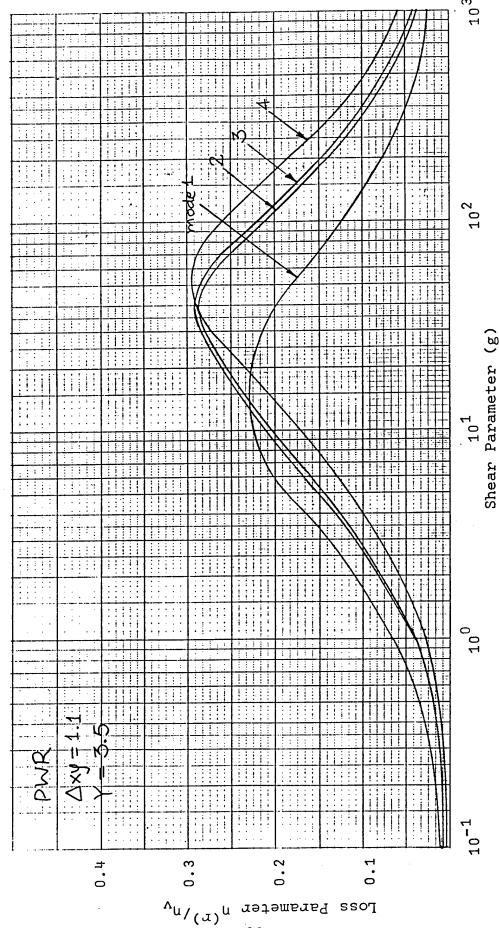
·						SHEAR F	SHEAR PARAMETER (9)	(6)				
		0.1	Steel F	Steel Face Sheets 1.		6.	30,	A	Aluminum Face	ace Sheets O.	1000	
MODE (r)	Freq. F (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq. (f,)	Loss rameter (n)	Freq.	Loss Parameter (n)	Freq.	oss ameter (n)	Freq.	Loss Parameter (n)
_	5643.	0.002	6094.	0.011	6258.	0.055	1245.	0.148	1506.	0.204	1701.	0.092
2	6062.	0.001	6587.	0.011	6765.	0.055	1354.	0.151	1658.	0.225	1899.	0.103
က	6553.	0.001	7480.	0.010	7679.	0.054	1544.	0.150	1911.	0.250	2236.	0.122
4	7442.	0.001	8757.	0.010	8979.	0.052	1817.	0.142	2255.	0.275	2700.	0.148
S.	8715.	0.001	10604.	0.009	1086.	0.047	2192	0.127	2709.	0.285	3320.	0.175
9	10556.	0.001	12895.	0.008	13184.	0.042	2664.	0.110	3251.	0.283	4055.	0.201
7	12841.	0.001	15690.	0.007	16016.	0.037	3244.	0.093	3892.	0.272	4912.	0.225
&	15629.	0.001	17662-	900.0	17990.	0.031	3641.	0.076	4254.	0.241	5277.	0.217
თ	17600.	0.001	18219.	0.005	18575.	0.032	3762.	0.073	4384.	0.235	5440.	0.217
10	18156.	0.001	18474.	900.0	18826.	0.034	3894.	0.078	4570.	0.261	5702.	0.224

## 3.2.5 PWR Boundary Conditions

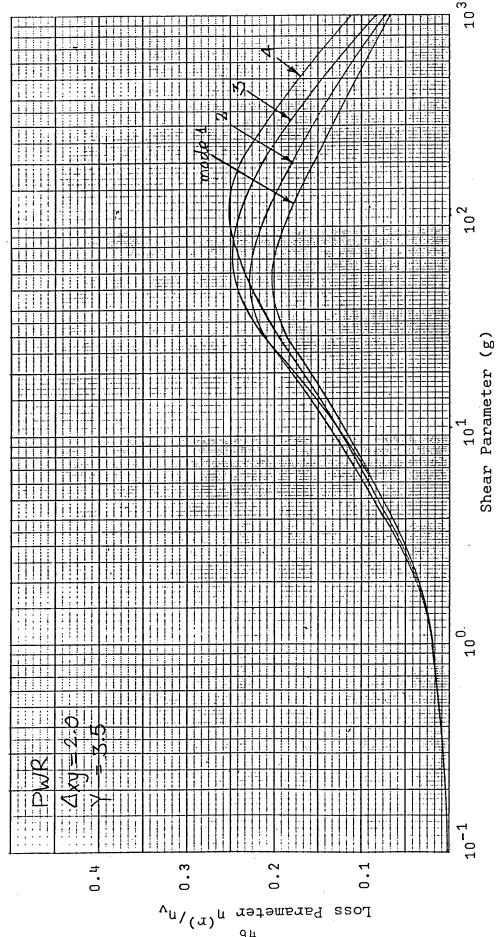
Damping as a function of the shear parameter for the first four modes of a rectangular sandwich plate with PWR boundary conditions and a geometry parameter of Y = 3.5 is shown in Figures 46 through 48.

Natural frequencies for sandwich plates with PWR boundary conditions are shown in Figures 49 through 51. Reference frequencies are given in Table 5.

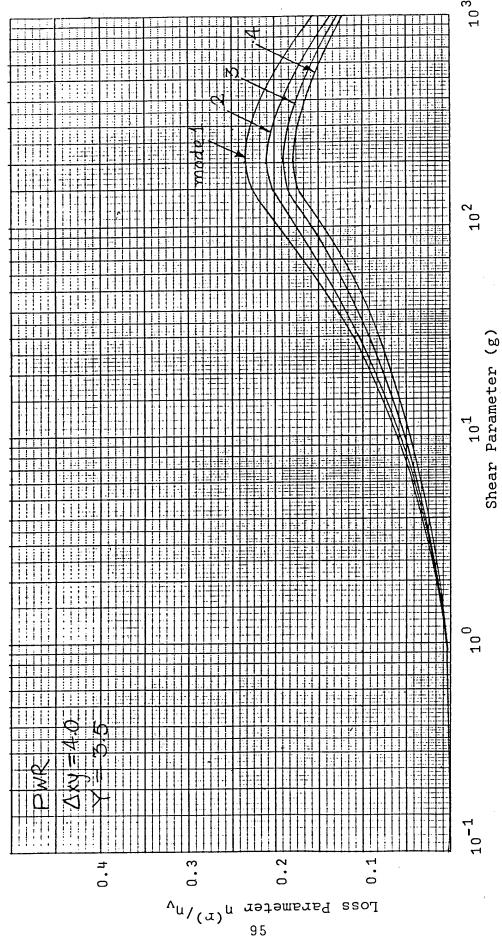
A tabular presentation of the data in Figures 46 through 51 is given in Tables 27 through 29, as well as results for the fifth and higher modes.



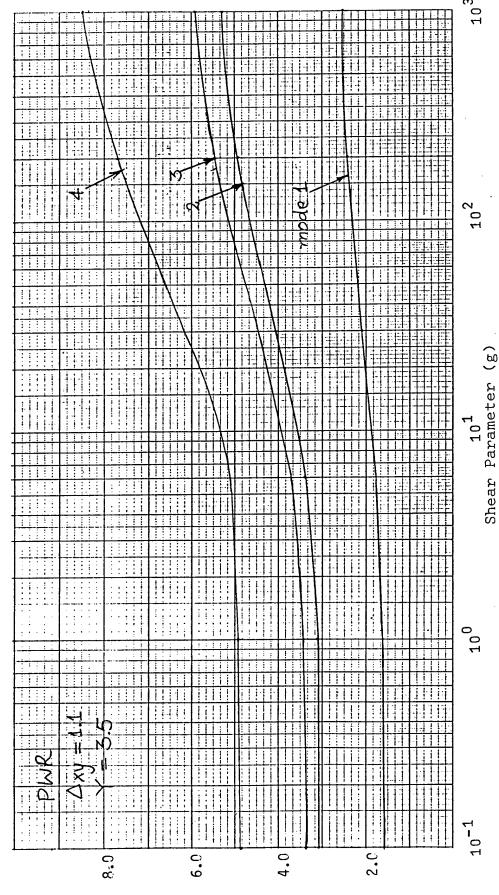
plate, 7 = 3.5 Damping of a sandwich rectangular boundary conditions,  $\Delta xy = 1.1, Y$ Figure 46



plate, = 3.5 sandwich rectangular Damping of a sandwich rec boundary conditions,  $\Delta xy$ 



plate, = 3.5 Damping of a sandwich rectangular boundary conditions,  $\Delta xy = 4.0$ , Y 48



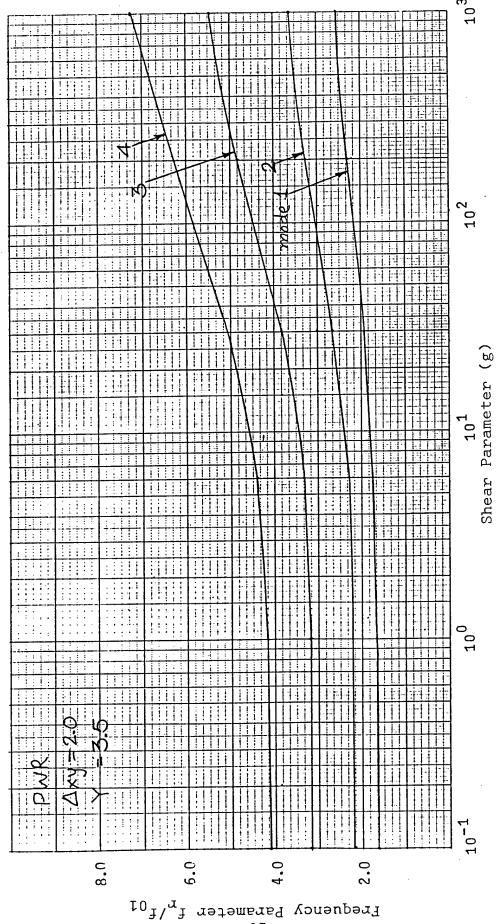
plate,

sandwich rectangular  $\Delta xy = 1.1$ , Y = 3.5

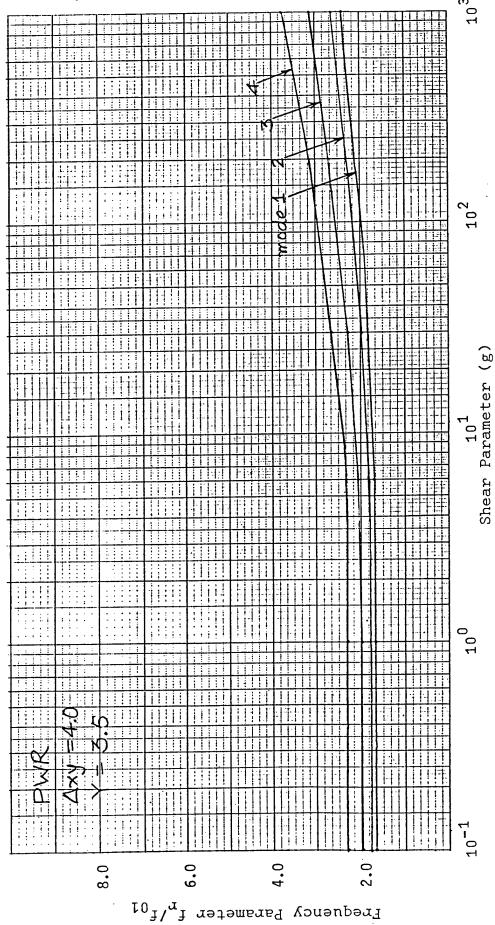
 $\Delta xy$ 

Natural frequencies of a PWR boundary conditions,

Erequency Parameter  $f_{\rm r}/f_{\rm 01}$ 



plate, sandwich rectangular  $\Delta xy = 2.0, Y = 3.5$  $\Delta xy$ Natural frequencies of a PWR boundary conditions, 50



plate, sandwich rectangular  $\Delta xy = 4.0, Y = 3.5$  $\Delta xy$ ൻ Natural frequencies of a PWR boundary conditions, 51 Figure

TABLE 27 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, elastically restrained rotation, zero shear) 전문 1.1 3.5 # # # E Boundary Condition Aspect Ratio (∆xy) Geometric Parameter

						SHEAR F	SHEAR PARAMETER (g)	R (g)				
		0.1	Steel 1	Face Sheets 1.0 V		6.		30.	Aluminum Face 200.	Face Sheets	ts 1000	
MODE (r)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter F (n)	req.	Loss Parameter (n)	Freq.	Loss Trameter (n)	Freq.	Loss rameter (n)	Freq (f,)	Loss Parameter (n)
_	835.	0.005	852.	0.043	929.	0.161	195.	0.218	1 .	0.094	238.	0.035
2	1612.	0.005	1645.	0.042	1789.	0.156	378.	0.247	463.	0.138	496.	0.043
က	1768.	0.005	1804.	0.041	1960.	0.155	414.	0.250	513.	0.155	557.	0.057
4	2571.	0.003	2604.	0.026	2764.	0.117	574.	0.236	724.	0.178	798.	0.064
rv.	2925.	0.003	2961.	0.025	3137.	0.115	649.	0.241	826.	0.197	918.	0.068
9	3351.	0.002	3388.	0.022	3573.	0.106	734.	0.235	942.	0.215	1065.	0.086
7	3847.	0.002	3883.	0.018	4063.	0.089	830.	0.212	1056.	0.216	1196.	0.084
œ	4101.	0.002	4138.	0.018	4327.	0.087	881.	0.209	1126.	0.225	1286.	0.093
ِ ه	4723.	0.002	4780.	0.022	5040.	0.092	1024.	0.197	1315.	0.229	1518.	0.100
10	5295.	0.001	5331.	0.014	5518.	0.070	1118.	0.182	1410.	0.249	1638.	0.113

TABLE 28 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, elastically restrained rotation, zero shear) 2.0 3.5 n n n Boundary Condition Aspect Ratio (Δxy) Geometric Parameter (Y)

	).	Loss Parameter (n)	0.067	0.070	0.084	0.113	0.113	0.392	0.400	0.409	0.401	0.385
	ts 1000.	Freq.	1 .	765.	1159.	1538.	1717.	1733.	2091.	2414.	2555.	3119.
	ace Sheets	Loss rameter (n)	0.154	0.179	0.215	0.223	0.251	0.341	0.348	0.349	0.351	0.347
	Aluminum Face 200.	Freq.	493.	694.	1026.	1334.	1471.	1493.	1779.	2010.	2143.	2591.
(a)		Loss Parameter (n)	0.188	0.211	0.210	0.191	0.187	0.367	0.387	0.399	0.407	0.416
SHEAR PARAMETER (g)	30	Freq.	410.	564.	811.	1072.	1154.	1203.	1438.	1596.	1751.	2137.
SHEAR F	0	Loss Parameter (n)	0.088	0.100	0.090	960.0	0.076	0.076	0.007	0.059	090.0	0.050
	6.0	req.	2056.	2766.	3980.	5236.	5713.	5923.	6082.	7146.	7969.	8667.
	Steel Face Sheets 1.0	Loss Parameter F (n)	0.019	0.022	0.0184	0.021	0.015	0.016	0.011	0.012	0.009	0.010
	Steel Fa 1.	Freg. (f <sub>r</sub> )	1971.	2634.	3809.	4978.	5495.	5700.	6931.	7715.	8460.	10324.
	.1	Loss Parameter (n)	0.002	0.002	0.002	0.002	0.002	0.002	0.001	0.001	0.001	0.001
	0	Freq. P (f <sub>r</sub> )	1953.	2607.	3776.	4925.	5452.	5657.	6890.	7667.	8419.	10259.
		(r)	_	2	т	. 4	S	9	7	ω	6	10

TABLE 29 MODAL FREQUENCIES AND MODAL LOSS FACTORS

(zero translation, elastically restrained rotation, zero shear) PWR 4.0 3.5 11 11 11 Boundary Condition Aspect Ratio (∆xy) Geometric Parameter (

						CHEAD	GITTE	(-)				
						SHEAK	PAKAME I EK	(6)				٠
	0	1	Steel F	Face Sheets 1.0		6.0	30.		Aluminum Face 200.	ace Sheets		1000.
MODE (r)	Freq. P	Loss Para <u>m</u> eter (n)	Freq. (f <sub>r</sub> )	Loss Para <u>m</u> eter (n)	Freq. (f <sub>r</sub> )	Loss Parameter (n)	Freq.	Loss Parameter (n)	Freq.	ss neter (r	Freg. (f,)	Loss Parameter (n)
_	6754.	0.001	6775.	0.006	6885.	0.035	1325.	0.079	1510.	0.178	1724.	0.123
2	7214.	0.001	7240.	0.007	7375.	0.039	1440.	0.093	1666.	0.191	1914.	0.126
m	8070.	0.001	8102.	0.008	8269.	0.045	1638.	0.105	1928.	0.210	2243.	0.134
4	9291.	0.001	9329.	0.008	9528.	0.044	1919.	0.110	2285.	0.235	2702.	0.154
ഹ	11100.	0.001	11146.	0.008	11386.	0.042	2304.	0.105	2755.	0.248	3321.	0.171
9	13358.	0.001	13411.	0.007	13692.	0.038	2785.	0.095	3515.	0.251	4061.	0.191
7	16146.	0.001	16206.	900.0	16529.	0.034	3378.	0.083	3977.	0.244	4927.	0.212
ω	16815.	0.001	16911.	0.009	17418.	0.046	3762.	0.114	4437.	0.210	5306.	0.182
6	17609.	0.001	17700.	0.008	18174.	0.041	3911.	0.099	4576.	0.200	5474.	0.180
20	18798.	0.001	18870.	0.007	19286.	0.032	4033.	0.072	4670.	0.238	5744.	0.189

### 3.3 EXAMPLE

As a guide in using the design charts, the sample problem discussed previously in Section 2.4 will be solved. The following data are given:

Boundary conditions = simply supported, unriveted (PTU) Base layer thickness,  $T_1 = 0.055$  inches Core layer thickness,  $T_2 = 0.0045$  inches Constraining layer thickness,  $T_3 = 0.055$  inches Viscoelastic shear modulus,  $\overline{G}_2 = 450 \text{ lbf/in}^2$ Base plate Young's modulus,  $E_1 = 10 \times 10^6 \text{ lbf/in}^2$ Constraining layer Young's modulus,  $E_3 = 10 \times 10^6 \, lbf/in^2$ Poisson's ratio of base layer,

 $v_1 = 0.3$ 

Poisson's ratio of constraining layer,

 $v_3 = 0.3$ 

Mass density of base layer,  $\rho_1 = 0.1 \text{ lbm/in}^3$  $= 2.59 \times 10^{-4} lbf-sec^2/in^4$ 

Mass density of constraining layer,

 $\rho_3 = 0.1 \text{ lbm/in}^3$ = 2.59 x 10<sup>-4</sup> lbf-sec<sup>2</sup>/in<sup>4</sup>

Mass density of viscoelastic layer,

 $\rho_2 = 0.035 \text{ lbm/in}^3$  $= 9.07 \times 10^{-5} lbf-sec^2/in^4$ 

Core material loss factor,  $\eta_{\rm v}$  = 0.3

Plate width, a = 10 inches

Plate length, b = 11 inches

The dimensionless variables that describe the plate are, in addition to n already given:

shear parameter 
$$g = \frac{\overline{G}_2}{T_2} \left[ \frac{1}{E_1 T_1} + \frac{1}{E_3 T_3} \right] b^2 (1 - v^2)$$
  

$$= \frac{450}{.0045} \left[ \frac{1}{10^7 \times .055} + \frac{1}{10^7 \times .055} \right] 11^2 \times (1 - 0.3^2)$$

$$= 40.0$$

D = sum of flexural stiffnesses of face sheets

$$= \frac{E_1 T_1^3}{12(1-v_1^2)} + \frac{E_3 T_3^3}{12(1-v_3^2)}$$

$$= \frac{10^7 \times .055^3}{12(1-0.3^2)} + \frac{10^7 \times .055^3}{12(1-0.3^2)}$$

$$= 304.7 \text{ lbf-in}$$

Y = geometry parameter

$$= \frac{(T_1 + 2T_2 + T_3)^2}{4D(1-v^2)} \left[\frac{E_1T_1E_3T_3}{E_1T_1 + E_3T_3}\right]$$

$$= \frac{(.055 + 2 \times .0045 + 0.55)^{2}}{4 \times 304.7 \times (1-0.3^{2})} \left[ \frac{10^{7} \times .055 \times 10^{7} \times .055}{10^{7} \times .055 + 10^{7} \times .055} \right]$$

$$= 3.51$$

Δxy = in-plane aspect ratio

= b/a

= 11.0/10.0

= 1.10

The normalizing frequency is found from

$$\rho = \text{total plate mass per unit area}$$

$$= \rho_1 T_1 + \rho_2 T_2 + \rho_3 T_3$$

$$= 2.59 \times 10^{-4} \times .055 + 9.07 \times 10^{-5} \times .0045 + 2.59 \times 10^{-4} \times .055$$

$$= 2.89 \times 10^{-5} \text{ lbf-sec}^2/\text{in}^3$$

$$f_{01} = \frac{1}{2\pi} \frac{D}{\rho} \left[ \left( \frac{\pi}{b} \right)^2 + \left( \frac{\pi}{a} \right)^2 \right]^2$$

$$= \frac{1}{2\pi} \frac{304.7}{2.89 \times 10^{-5}} \left[ \left( \frac{\pi}{11} \right)^2 + \left( \frac{\pi}{10} \right)^2 \right]$$

$$= 93.16 \text{ Hz}$$

Figure 6 gives damping as a function of g for PTU boundary conditions, Y = 3.5, and  $\Delta xy = 1.1$ . Entering the chart with g = 40 gives

$$\frac{\eta^{(1)}}{\eta_{v}} = 0.240$$
 for mode 1

 $\frac{\eta^{(2)}}{\eta_{v}} = 0.336$  for mode 2

 $\frac{\eta^{(3)}}{\eta_{v}} = 0.345$  for mode 3

 $\frac{\eta^{(4)}}{\eta_{v}} = 0.331$  for mode 4

or, since  $\eta_v = 0.3$ 

$$\eta^{(1)} = 0.240 \times 0.3 = 0.072$$
 $\eta^{(2)} = 0.336 \times 0.3 = 0.101$ 
 $\eta^{(3)} = 0.345 \times 0.3 = 0.104$ 
 $\eta^{(4)} = 0.331 \times 0.3 = 0.099$ 

Natural frequencies are found by entering Figure 21 (applicable for PTU boundary conditions, Y = 3.5, and  $\Delta xy = 1.1$ ) with g = 40 to obtain

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$$\frac{f_{1}}{f_{01}} = 1.75$$

$$\frac{f_{2}}{f_{01}} = 3.60$$

$$\frac{f_{3}}{f_{01}} = 4.08$$

$$\frac{f_{4}}{f_{01}} = 5.63$$

Then, using the calculated reference frequency of  $f_{O1} = 93.2 \text{ Hz}$  gives

$$f_1 = 1.75 \times 93.16 = 163.0 \text{ Hz}$$
  
 $f_2 = 3.60 \times 93.16 = 335.4 \text{ Hz}$   
 $f_3 = 4.08 \times 93.16 = 380.1 \text{ Hz}$   
 $f_4 = 5.63 \times 93.16 = 524.5 \text{ Hz}$ 

These values may be compared with results given for the same problem in Appendix A (the raw NASTRAN output) and in Section 4.3 (the closed form solution).

# 4.0 CLOSED FORM SOLUTION FOR HIGH-ORDER MODES

#### 4.1 THEORY

Structures built up from plates always have numerous highorder modes of vibration involving flexure of local sections.
Calculation of all these modes with a single finite element
model, while theoretically possible, is neither practical nor
efficient. However, such modes can be important if high frequency excitation is present. Therefore, a method is proposed
for designing a damping treatment to suppress modes of this type
which avoids the cost of calculating the properties of a large
number of essentially similar modes. The method is usable for
either add-on or integral damping but is most likely to be used
for add-on treatments.

The method is based on the fact that the higher order mode shapes of rectangular plates tend to be sinusoidal in both inplane directions except near the boundaries. Boundary conditions have little effect on the higher order mode shapes over most of the plate area. This is true for either classical uniform plates or three-layer sandwich plates formed by adding a constrained layer damping treatment to a uniform plate. This property leads to a useful relationship between natural frequencies and modal loss factors. When modal loss factors are plotted against modal frequencies for a sandwich plate, the relationship is essentially independent of boundary conditions so long as the boundary conditions themselves are non-dissipative [10].

In the present case, we are interested in damping a number of modes over a fairly wide band of frequencies. The exact value of natural frequency for each of the many plate modes is not of particular importance. We may therefore calculate the relation between modal loss factor and modal frequency based on any convenient set of boundary conditions. The curve plotted for any other boundary condition would be formed by points at different frequencies but would still fall on or near the first, particularly for higher modes. By choosing a set of boundary conditions

which lead to a simple closed form solution, we may produce the plot of damping vs. frequency quite easily for any given material properties and plate cross-section. A few trials will usually be sufficient to find an appropriate add-on treatment based on the size, thickness, and material of the base plate and the desired frequency range.

The most convenient set of boundary conditions are those where all four sides of the plate are simply supported and shearing of the viscoelastic core is unrestrained. In this case the mode shapes are sinusoidal all the way to the edges of the plate. Two closed form solutions for this case are available in the literature [8,10]. The former solution, due to Abdulhadi, has been used in this work because the latter could not be made to produce results in agreement with MSC/NASTRAN-MSE even for small values of the core material loss factor. The Abdulhadi solution did produce good agreement for core loss factors small compared to unity. It diverged somewhat as the core loss factor approached unity, as shown in Section 2 of this report and in previous work by the authors [1].

The closed form solution from Ref. [8] for the natural frequency and modal loss factor of a simply supported rectangular sandwich plate is, using the notation of Eq. (6-8):

$$p_{r} = \left[\frac{\alpha_{r}^{2}D}{\rho}\right] \left[1 + \frac{\frac{\sigma^{2}K_{e}\alpha_{r}}{\overline{G}_{2}} + 1 + \eta_{v}^{2}}{\left(\frac{\sigma^{2}K_{e}\alpha_{r}}{\overline{G}_{2}} + 1\right)^{2} + \eta_{v}^{2}}\right]$$
(28)

$$\eta^{(r)} = \frac{\eta_{v}(\frac{T_{2}^{K}e^{\alpha_{r}}}{\overline{G}_{2}}) (\frac{K_{e}\delta^{2}}{D})}{N_{1r} + N_{2r}}$$
(29)

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where

$$N_{1r} = \left(\frac{T_2^{K_e \alpha_r}}{\overline{G}_2} + 1\right)^2 + n_v^2$$
 (30)

$$N_{2r} = \frac{\delta^2 K_e}{D} \left(1 + \eta_v^2 + \frac{T_2 K_e^{\alpha} r}{\overline{G}_2}\right)$$
 (31)

$$D = \frac{E_1 T_1^3}{12(1-v_1^2)} + \frac{E_3 T_3^3}{12(1-v_3^2)}$$
 (32)

$$\delta = \frac{\mathbb{T}_1}{2} + \frac{\mathbb{T}_3}{2} + \mathbb{T}_2 \tag{33}$$

$$\rho = \rho_1 T_1 + \rho_2 T_2 + \rho_3 T_3 \tag{34}$$

$$K_{e} = \frac{E_{1}T_{1}E_{3}T_{3}}{(1+v_{1})[E_{1}T_{1}(1-v_{3}) + E_{3}T_{3}(1-v_{1})]}$$
(35)

p<sub>r</sub> = radian frequency of the r'th mode

 $\eta^{(r)}$  = structural loss factor of the r'th mode

 $\eta_{vr}$  = material loss factor of the viscoelastic core

 $T_2$  = thickness of the core layer

 $T_1, T_3$  = thicknesses of the face sheets

E<sub>1</sub>,E<sub>2</sub>,E<sub>3</sub> = Young's moduli of the materials of the three layers

 $v_1, v_2, v_3$  = Poisson's ratios for the three layers

 $G_2^*$  = complex shear modulus of the core =  $G_2(1 + i n_v)$ 

$$\alpha_{\mathbf{r}} = \left(\frac{\mathbf{n}\pi}{\mathbf{n}}\right)^2 + \left(\frac{\mathbf{n}\pi}{\mathbf{n}}\right)^2$$

a,b = in-plane dimensions of the plate

m,n = integers

The use of simply supported plate solutions for designing a damping treatment for a plate with any non-dissipative boundary conditions may seem like a gross approximation. However, the end purpose of the damping must be kept in mind. Local plate modes of an all-welded structure without any damping treatment would typically have loss factors on the order of .001. Loss factors predicted by the simple method described here are on the order of .1, even without any extensive searching for an optimum damping treatment. If boundary condition effects change the loss factors by a factor of three, the end conclusion regarding the damping treatment will remain the same; namely, that it produces a substantial (factor of 30) reduction in resonant response to periodic input forces. Complex eigenvalue solutions for sandwich beams have shown that the variation of damping with boundary conditions is small for higher modes and generally less than a factor of three for low order modes [1].

### 4.2 SOFTWARE IMPLEMENTATION

The equations given above have been implemented in an interactive program called SPLT61 [11]. The program allows a designer to quickly evaluate a number of possible constrained layer treatments with negligible cost for computing. The input to the program includes flexural wavelength in each direction. These are usually set equal to twice the plate dimensions, i.e., the exact value for a simply supported plate. The output is the exact (i.e., closed form, complex eigenvalue) solution for modal frequencies and loss factors of a simply supported sandwich plate. As noted above, this frequency-damping relationship is also correct in the limit of increasing mode number for other boundary conditions.

The validity of using a solution for a simply supported sandwich plate to predict damping for other boundary conditions. was tested as follows. The SPLT61 program was used to obtain modal damping as a function of modal frequency for a number of different values of the shear parameter. The shear parameter was varied by changing G2, the core shear modulus. The SPLT61 results represented an exact solution for a simply supported plate, (i.e., PTU boundary conditions). A similar analysis was performed for a plate with PLR (fixed) boundary conditions using the modal strain energy method. A comparison of results is shown in Figures 52 through 55. The damping parameter is plotted vs. normalized modal frequency for various values of the shear para-The frequency is normalized on a reference frequency  $f_{01}$ as described in a previous section. The same information is shown in dimensional form in Figures 56 through 60. It may be seen that the results for the drastically different boundary conditions do, in fact, converge at high frequency. Significantly, the rate of convergence depends on the shear parameter.

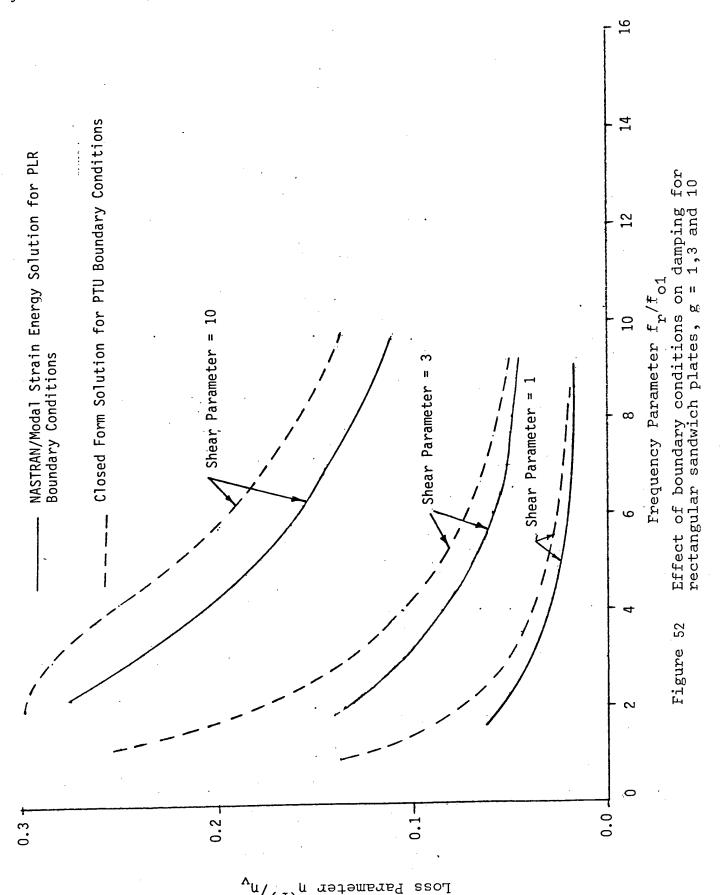
### 4.3 EXAMPLE

The use of the SPLT61 program is illustrated below. The physical situation being analyzed is the same one treated using the NASTRAN modal strain energy method in Section 2.4 and using the design charts in Section 3.3. Since true simply supported boundary conditions are assumed, and the viscoelastic loss factor is small, the SPLT61 solution agrees closely with the NASTRAN/MSE results.

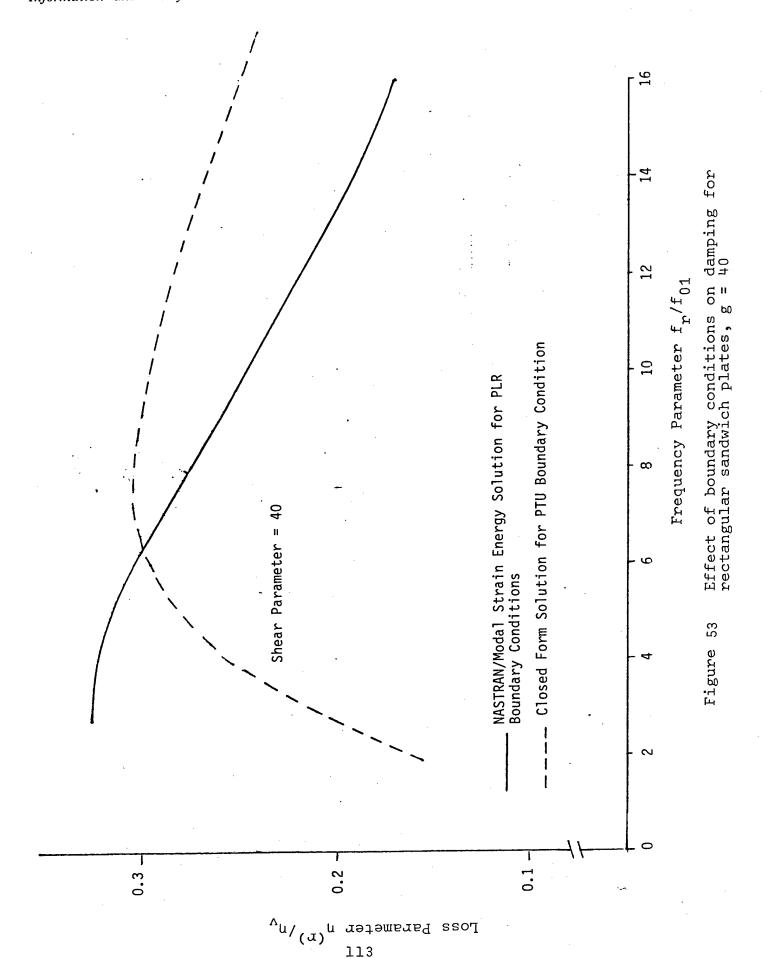
The program begins by displaying default values for all the physical parameters describing a three-layer, rectangular sandwich plate. The user is prompted to change any or all of the values, to store the entire list in a disc file, or to proceed with the calculations. He has the option of resetting the entire list of values to a set previously stored on disc. Once the user is satisfied with the input data, he commands the program to proceed. It then calculates a table of natural frequencies and

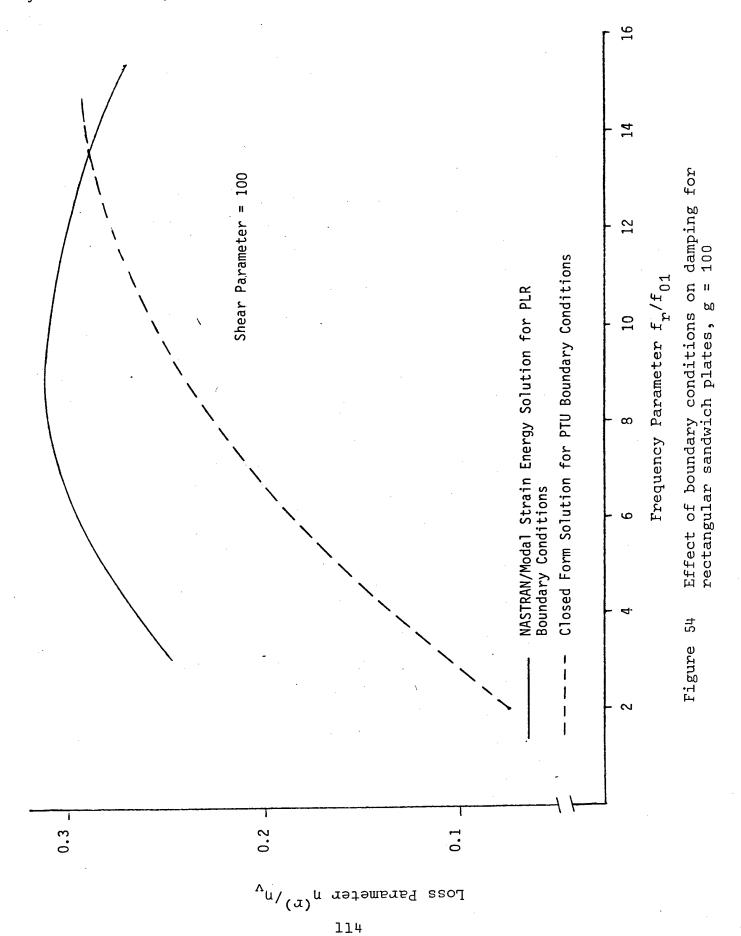
modal loss factors for the first ten allowable values of wavenumber in each in-plane direction (i.e., one hundred normal modes). The resulting table, along with the input parameter list, is written to a disc file in ASCII format to be printed later. The modes calculated are not necessarily the lowest one hundred, but will always contain the lowest ten.

The output file for the sample case is shown in Table 30. The loss factors are given directly rather than being normalized on the core material loss factor. They must therefore be divided by that value (0.3 for the sample case) for comparison with results from the other two methods.



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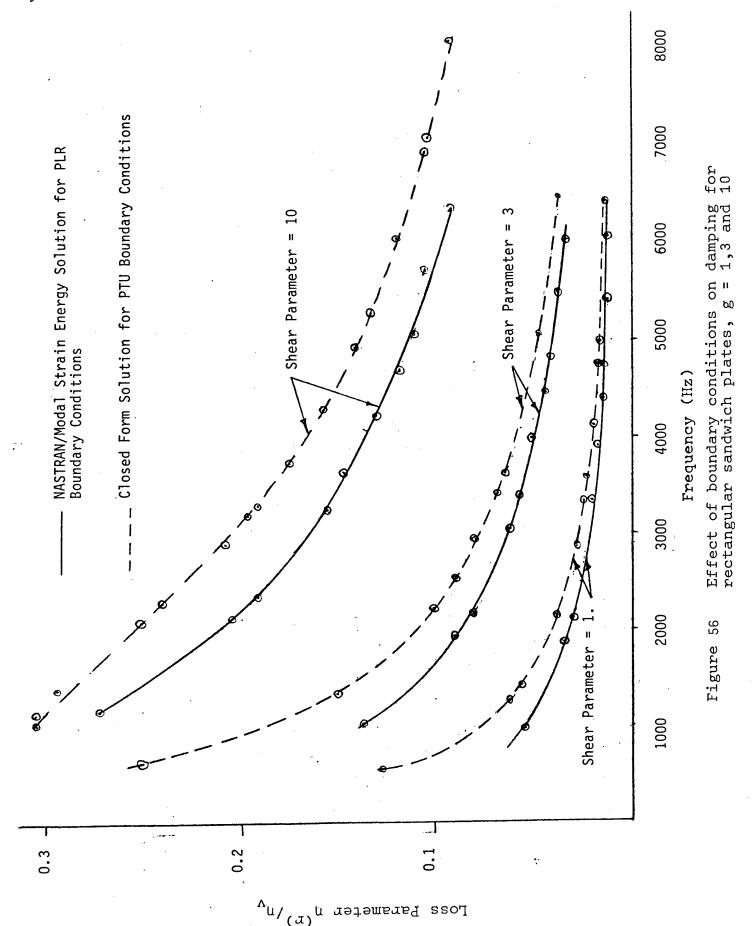


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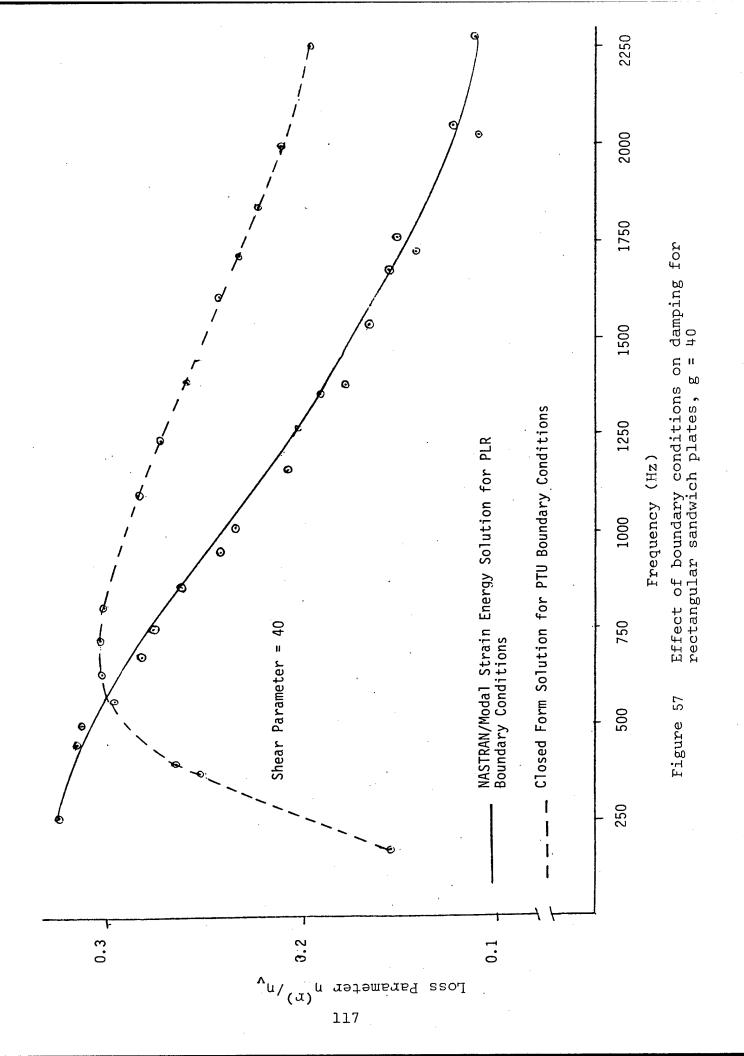
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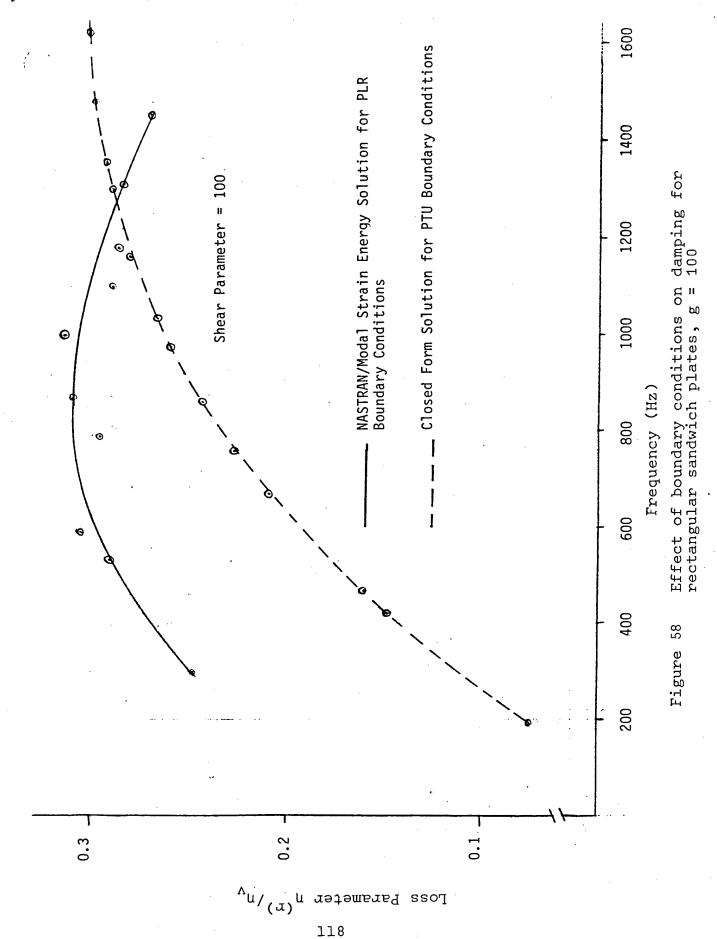
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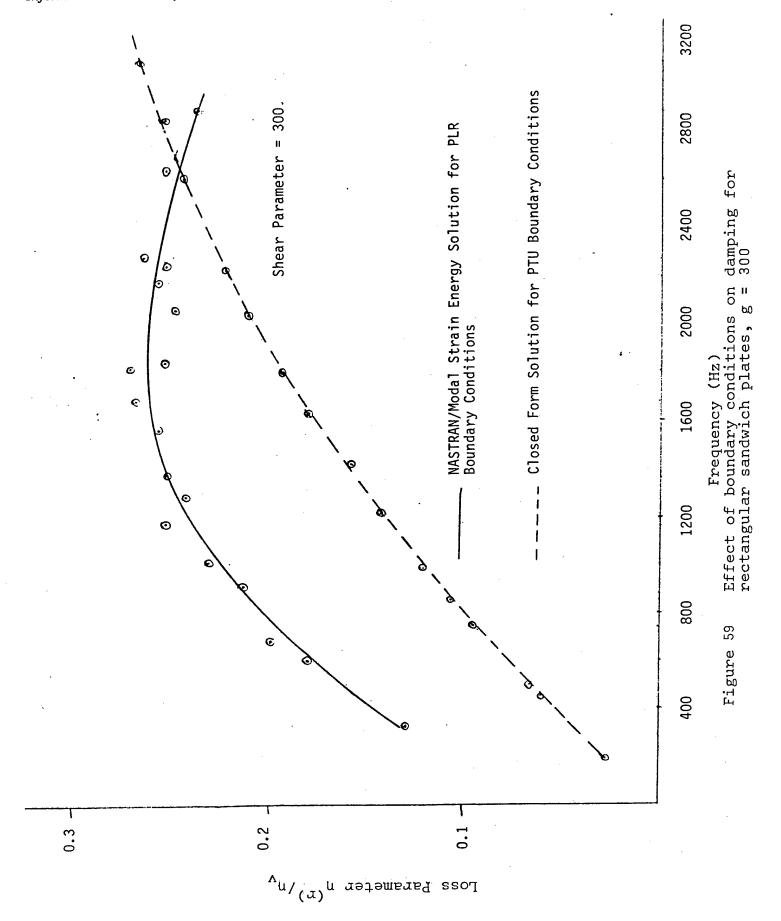
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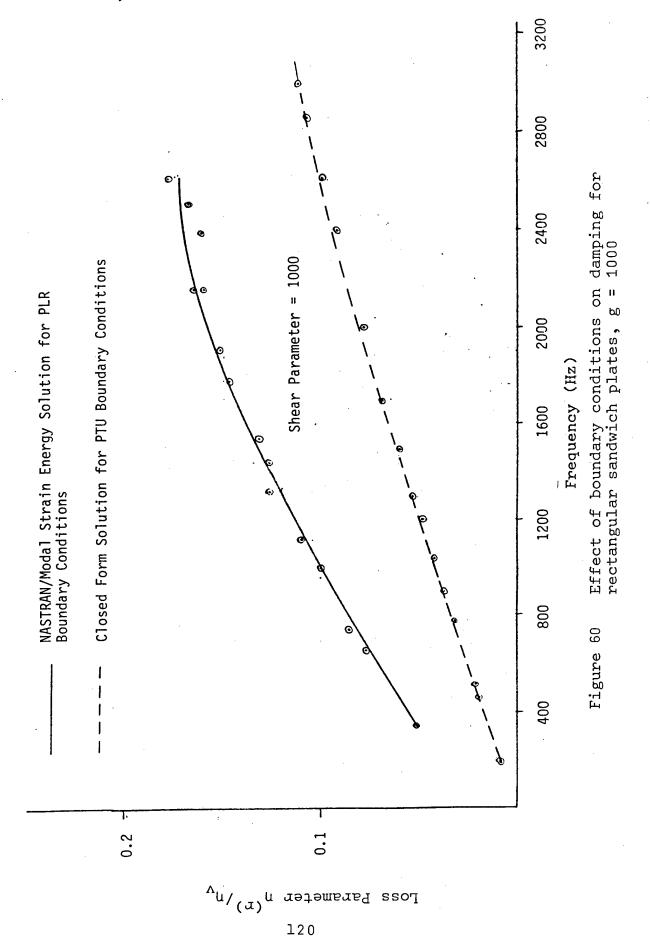


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RESULTS FROM CLOSED FORM SOLUTION FOR SIMPLY SUPPORTED RECTANGULAR PLATE TABLE 30

APPRCXIMATE LOSS FACTORS FOR A RECTANGULAR THREE LAYER SANDWICH PLATE

PRUGRAM SPLT61

07-JAN-83

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#### 5.0 SUMMARY AND CONCLUSIONS

Three methods have been presented for the dynamic analysis and design of viscoelastically damped sandwich plates. The methods are complementary in that each represents a different trade-off of accuracy, generality, and cost of use. The theoretical basis of each has been described along with sample problems.

The most general of the three is the modal strain energy method implemented in MSC/NASTRAN. It is fairly new, having been developed primarily by the authors of this report. It can accommodate virtually any combination of plate geometry and boundary conditions. The method is by no means limited to sandwich plates, although it has seen extensive application there due to the efficiency of this construction for vibration damping. The price for the accuracy and generality of MSE is that the user must be fluent in NASTRAN and must prepare and run a finite element model for each candidate design.

A simplified version of MSC/NASTRAN-MSE for rectangular sandwich plates is presented in the form of design charts derived from a large number of MSC/NASTRAN-MSE runs. They allow the user to rapidly obtain values for modal loss factors and modal frequencies of sandwich plates with various boundary conditions Plate geometry and material properties are specified in terms of dimensionless groups to allow the maximum information to be conveyed by each chart.

The simplest of the three methods is based on the use of a closed form solution that is strictly applicable only to simply supported rectangular sandwich plates. It is shown that the solution may be used with other boundary conditions to obtain damping estimates of useful accuracy for higher order modes. The method involves negligible costs for computation and has been implemented in an interactive Fortran program.

The latter two methods are applicable only to single rectangular plates rather than to assemblages built up of plate

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elements. They are nonetheless useful in that a designer often seeks to increase the damping of local modes of individual plate sections. The modal strain energy method, when implemented in NASTRAN, is quite general and will readily accommodate built-up structures with integral damping.

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# APPENDIX A

SAMPLE INPUT AND OUTPUT FOR

NASTRAN/MODAL STRAIN ENERGY ANALYSIS

OF A SANDWICH PLATE

Sample NASTRAN input is given for the following case:

Boundary condition = PTU (zero out-of-plane translation, zero moment, unrestrained core shear)

Face sheet thicknesses (equal),  $T_1 = T_3 = 0.055$  in.

Core layer thickness,  $T_2 = 0.0045$  in.

Viscoelastic shear modulus,  $\overline{G}_2 = 450 \text{ lbf/in}^2$ 

Face sheet Young's moduli (equal),  $E_1 = E_3 = 10^7 \text{ lbf/in}^2$ 

Poisson's ratio of face sheets (equal),  $v_1 = v_3 = 0.3$ 

Poisson's ratio of core layer,  $v_2 = 0.49990$ 

Mass density of face sheets (equal),

$$\rho_1 = \rho_3 = 2.59 \times 10^{-4} \text{ lbf-sec}^2/\text{in}^4$$

Mass density of core layer,  $\rho_2 = 9.07 \times 10^{-5} \text{ lbf-sec}^2/\text{in}^4$ Plate in-plane dimensions, a x b = 10 x 11 inches

The entire bulk data deck is listed for clarity although most of it was produced automatically by a mesh generator. Executive and case control decks are also listed. The grid and the numbering system is illustrated in Figure A-1.

Output listed for the sample case includes the first page of the eigenvalue table, mode shapes for the first four modes, and strain energy distributions for the first four modes. Set 99 includes all the solid elements used to model the core. Thus, the "percent of total" figure printed out is (after dividing by 100) exactly the strain energy fraction  $V_V^{(r)}/V^{(r)}$  of Eq. 4, which equates to the loss parameter  $\eta^{(r)}/\eta_V$ .

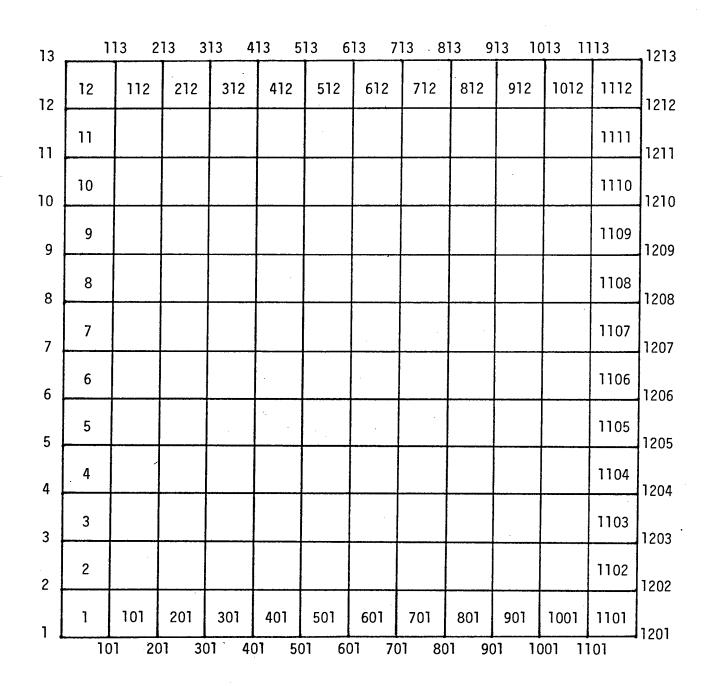


Figure A-l Finite element grid for modal strain energy analysis of rectangular sandwich plate. Upper face sheet showing partial grid numbering and QUAD4 element numbering.

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10001	10101	10201	10301	10401	10501	10601	10701	10801	10901	11001	11101

Figure A-2 Finite element grid for modal strain energy analysis of rectangular sandwich plate.

Viscoelastic core layer showing partial HEXA element numbering.

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Figure A-3 Finite element grid for modal strain energy analysis of rectangular sandwich plate.

Lower face sheet showing grid numbering and QUAD4 element numbering.

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		A L E II G E	AVALUES		
EXTRACTION ORDER	EIGENVALUE	RADIANS	CYCLES	GENERAL 1 ZED PASS	GENERAL12EU Stiffness
13	9561E	.048600E+	1.668898E+02	0	8.729356E+02
135	1975E	2.178296E+03	3.466866E+02	8.439574E=04	4.004556E+03
761	980	57201E+	5,3431516+02	٠.	8,749392E+03
135	9947E		060328E+0	740813	1,122376E+04
136	71938E	.326590E+	E+0	3508	1.434561E+04
137	76467E	74902E+	7.758648E+02	.008836	97466E+
1 36	725/1E		6.638241E+02	8.904302E=04	3.211574E+04
1 4 1	34321E	583556E+	1.047805E+03	7.771628E-04	0
143	.843452E	959491E+	1.107637E+03	8.765702E-04	0
142	5.065117E+07	6963E+	1.132700E+03	9.138648E-04	.628832E+0
140	6.246809E+07	7.903676E+03	1.257909E+03	8.964685E-04	9 9
	.711511E	6.781521E+03	. 1639/622E+05		7 420210E404
- t	3570100			7 2405625=04	.148064F+0
1 20	0084686	0042255	1.5982786+03		553929E+0
129	1,066431E+08	032682		7.008227E-04	473792E+0
128	1.307702E+08		1.820013E+03	6.205890E-04	115455
127	1,388329E+08	1.178274E+04	1-875281E+03	S.444824E-04	•
126	1.4352326+08	1980126+04	1.906695E+03	0.0	0.00
125	1.4524/2E+U8	1.2847925+04	1.710112E+U3		0.0
121	1.753908E+08	1 . 324352E+04	2.107771E+03	• •	
122	2.044341E+08	98		0.0	0.0
11.5	307806	1.519146E+04	2,417796E+03	0.0	0.0
120	3459986	1.5316656+04		<b>.</b>	D 0
151	2.347524E+08	1. U3. 1. 10. 11. 10. 10. 10. 10. 10. 10. 10.	Z.438513E+03		) • • • • • • • • • • • • • • • • • • •
2.5	941704	1.7122345+04	725087E+0	•	
116	070612	1.752316E+04	.786898E+0		0.0
115	394016	1,842286E+04	932089E+0	•	0.0
<b>7</b> :	3,7342446+08	2419E+0	075541E+	٠	
M 1	3.414544E+U8	1.9/6535E+64	3.148934E+U3		) • O
3	505560	2.1726315+04		0.0	0.0
110	.989128E+0	23	3.554941E+03	0.0	0.0
106	ó	92758E	649038E+0	•	
100	.273597E+0	2.296431E+04	3.654884E+03	•	0.0
107	.721795E+0	9 9	3.807030E+03	0.0	0.0
9 9	.036311E+0	46043/E		0.0	•
n <	2 9	375004E+	4 0504805404	•	•
70	783481F+0	544750E+0	45209F	• • •	•
102	231278E+0	.869020E+	566188E+0	0.0	
101	0	_	957378E+0		0.0
	9.883113E+08	3.143742E+04	003420E+0	0.0	0.0
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0.00	-	ی	.01833E-0	-	1,833453E+	1.937617E	.746285E		,
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0.00000000000000000000000000000000000	<b>&gt;</b> 0	ں و	0.0	2.0645655+05	0.0	•	9.989451E=0	•	
1.094206=03   1.01256=04   1.01356=01   1.02406=01   1.01556=01   1.015616=01   0.01	0	ی و	119491E-0	1.779843F-03	4.3293245	•	80393	0.0	
1,20425046=03   1,0121216=03   1,8441395=01   1,5261646=01   1,5365116=01   0,000000000000000000000000000000000	0	ى د	859026E-0	1.403661E-03	6.121512E-	, ,		0	
1,22001EE-03	0	ی ر	.094250E-0	1.012121E-03	7.	1.227046E-01		0.0	
1.22061EE	0	ی	.228784E-0	5.913254E-04	8.3	.568113E-0	3	0.0	
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6 5.55842E-04 -1.31317E-03	9 0	ه.و	0.220611E=0	-5.538113E -04	ם פ	.555446E	<u>.</u>	2.0	
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C	-	ی	.888016E-0		4.3	,138122E		0.0	
C 1748506=04 2.128997E=03 0.0 2.749128E=01 -9.465509E=04 0.0 C 1748506=04 2.132899E=03 4.813138E=01 2.44724E=01 -9.465519E=02 0.0 C 1748506=04 2.132899E=03 4.813139E=01 2.44724E=01 -9.465519E=02 0.0 C 1748506E=04 1.81319E=03 4.81451E=01 1.371094E=01 -9.455119E=02 0.0 C 2.55943E=04 1.8131888E=03 1.813179E=01 1.371094E=01 -9.455119E=02 0.0 C 5.59943E=04 1.8131888E=03 1.813179E=01 1.371094E=01 -7.25119E=02 0.0 C 5.59943E=04 1.813188E=03 1.8131799E=01 1.371094E=01 -7.25119E=02 0.0 C 6.113621E=04 1.813178E=03 9.85978E=01 1.371094E=01 -7.25119F=02 0.0 C 6.113621E=04 1.813178E=03 9.8978E=01 1.371094E=01 -7.25170E=02 0.0 C 7.929276E=04 1.813178E=03 9.8978E=01 1.371094E=01 -7.25170E=02 0.0 C 9.29276E=04 1.813178E=03 9.89781E=01 -2.780177E=01 -7.25170E=02 0.0 C 9.29276E=04 1.813178E=03 9.89781E=01 -2.780177E=01 -7.25170E=02 0.0 C 9.29276E=04 1.813178E=03 9.89781E=01 -2.780177E=01 -7.25170E=02 0.0 C 9.29276E=04 1.81372E=03 9.89781E=01 -2.780177E=01 -7.2517318E=02 0.0 C 9.29276E=04 1.81372E=03 9.89781E=01 -2.780177E=01 -7.2517318E=02 0.0 C 9.29276E=05 1.84778E=03 7.001881E=01 -2.780177E=01 -7.77097E=05 0.0 C 9.29276E=05 1.84778E=03 7.001881E=01 -2.888776E=01 -1.18778E=05 0.0 C 9.29276E=05 1.84778E=03 7.001881E=01 -2.888776E=01 -1.18778E=05 0.0 C 9.282756E=05 1.84778E=03 7.001881E=01 -7.481776E=02 0.0 C 9.282756E=05 1.84778E=03 7.001881E=01 -7.481776E=02 0.0 C 9.282756E=05 1.84778E=03 7.001881E=01 -7.481776E=02 0.0 C 9.287576E=05 1.88778E=03 7.001881E=01 -7.481776E=01 0.0 C 9.287576E=05 1.84778E=03 7.001881E=01 -7.481776E=01 0.0 C 9.287576E=05 1.88778E=03 7.001881E=01 -7.487776E=01 0.0 C 9.287576E=05 1.88778E=03 7.001881E=01 -7.487776E=01 0.0 C 9.287576E=05 1.88778E=03 7.001881E=01 -7.487776E=01 0.0 C 9.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-	٠	.897860E-0		2.240401E	.371343E	٠	0.0	
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3,45670EE	<b>&gt;</b> <	. و	7.85045-0	C - 3C 00 74 C	0.0	103160F	145424		
6 6.59938E=04 1.58948F=03 6.8568E=01 1.53095E=01 -7.92531F=02 0.0 6 6.59938E=04 1.58098E=03 9.5694F=01 1.57095E=01 -7.92531F=02 0.0 6 6.59938E=04 1.510737E=05 9.5697E=01 1.51095E=04 -8.119176E=02 0.0 6 6.59938E=04 1.510737E=05 9.5697E=01 1.51095E=04 -8.119176E=02 0.0 6 6.113821E=04 1.59737E=05 9.5697E=01 1.57200FE=02 7.7953847E=02 0.0 6 6.113821E=04 1.59737E=05 9.59735E=01 1.57200FE=01 -7.251077E=02 0.0 6 1.13821E=04 1.59737E=05 9.59735E=01 1.57200FE=01 -7.251077E=02 0.0 6 1.13821E=04 1.59737E=05 9.59735E=01 1.57200FE=01 -7.251077E=02 0.0 6 1.13821E=04 1.59737E=05 9.562346E=01 -7.26974E=01 1.672074E=02 0.0 6 1.25941E=04 -2.23772E=03 0.0 7.265941E=01 -2.26596E=02 0.0 6 1.265941E=04 -2.159508E=03 2.583461E=01 -2.164738E=02 0.0 6 1.265941E=04 -2.159508E=03 2.583461E=01 -2.164738E=02 0.0 7.265941E=04 -2.159508E=03 2.583461E=01 -2.164738E=02 0.0 6 1.265941E=04 -2.16478E=03 0.0 7.265941E=01 -2.164449E=05 0.0 7.265969E=03 0.0 7.265969E=01 -2.166999E=03 0.0 7.265969E=01 -2.169999E=03 0.0 7.265969E=01 -2.169999E=03 0.0 7.266969E=01 -2.169999E=03 0.0 7.266969E=01 -2.164419E=02 0.0 7.266969E=01 -2.164419E=02 0.0 7.266969E=01 -2.164419E=02 0.0 7.266969E=01 -2.164419E=02 0.0 7.266969E=01 -2.164419E=02 0.0 7.266969E=01 -2.16419E=02	<b>&gt;</b>	ون و :	456776E=0	2 0015205	000000000000000000000000000000000000000	180571F	1618675	•	
6 6.23434E=04	100 100 100		985666E=0	1.587467E=03	6.836689	1.932122E-01	887521E	0	
6 6,559348E-04	505		.248398E-0	1.13984E-03	-8,3602	-1.370954E+01-	.2553196	0.0	
6 6. 488398E=04	206	ی	.559364E-0	6.220292E-04	6	.182510E-0	.932923E	0.0	
6 6.113821E-04 -1.07502FE-03 6.812730E-01 -1.372208E-01 -7.20107FE-02 0.0  6 1.625941E-04 -1.6551704E-03 6.812730E-01 -1.372208E-01 -7.20107FE-02 0.0  6 1.625941E-04 -1.67622C-03 4.829351E-01 -2.38607E-01 -7.20107FE-02 0.0  6 0.0 2.293772E-03 0.0 2.869351E-01 -2.38607E-01 -2.107133E-02 0.0  6 0.0 2.293772E-03 0.0 2.86935E-01 -2.36659EE-02 0.0  6 0.0 2.293772E-03 0.0 2.869451E-01 -2.36690EE-06 0.0  6 0.0 2.286690EE-06 2.003177E-03 2.594461E-01 -2.369690E-06 0.0  6 0.0 3.10648E-06 2.003177E-03 5.006485E-01 2.008934E-01 -1.13319E-05 0.0  6 0.0 3.10648E-06 2.003177E-03 5.006485E-01 2.008934E-01 -1.3319E-05 0.0  6 0.0 3.10648E-06 2.003177E-03 5.006485E-01 -1.41742E-05 0.0  6 0.0 3.10648E-06 2.003177E-03 5.006485E-01 -1.3319E-05 0.0  6 0.0 3.10648E-06 2.003177E-03 5.006485E-01 -1.01753E-05 0.0  6 0.0 3.10648E-06 2.003177E-03 5.006485E-01 -1.01753E-05 0.0  6 0.0 3.106485E-05 -1.60788E-03 0.008934E-01 -1.01753E-05 0.0  7 0.0 3.106485E-05 -1.60788E-03 0.008934E-01 -1.008973E-05 0.0  8 0.0 3.106485E-05 -1.60788E-03 0.008934E-01 -1.008973E-05 0.0  9 0.0 3.106485E-05 -1.64784E-03 0.008934E-01 -1.008973E-05 0.0  9 0.0 3.106485E-05 -1.647845E-03 0.008934E-01 -1.008973E-05 0.0  9 0.0 3.106485E-04 0.00842E-03 0.008934E-01 -1.008973E-05 0.0  9 0.0 3.106485E-04 0.00842E-03 0.009682E-01 -1.008973E-05 0.0  9 0.0 3.106485E-04 0.00842E-03 0.009682E-01 -1.008973E-05 0.0  9 0.0 3.106485E-05 -1.64784E-03 0.009682E-01 -1.008973E-05 0.0  9 0.0 3.106485E-04 0.00842E-03 0.009682E-01 -1.008973E-03 0.0  9 0.0 3.106485E-04 0.00842E-03 0.009682E-01 -1.008973E-03 0.0  9 0.0 3.106485E-04 0.00842E-03 0.009682E-01 -1.008973E-03 0.0  9 0.0 3.106485E-05 0.0 3.106482E-03 0.009682E-01 -1.00893E-03 0.0  9 0.0 3.106485E-05 0.0 3.106482E-03 0.009982E-03 0.00998	507	<b>.</b>	.539243E-0	8073	9.659785E-01	.016036E-0	.119176E	0.0	
4,800135E=04         -1,551704E=03         6,822730E=01         -1,93238E=01         -5,895426E=02         0           1,625941E=04         -2,15070E=03         2,502346E=01         -2,48517733E=02         0           0,0         -2,39372E=03         2,502346E=01         -2,4851733E=02         0           0,0         -3,15070E=03         2,502346E=01         -2,4851733E=02         0           0,0         -3,160474E=01         -2,48540E=02         0         0           0,0         -3,160474E=01         -2,48640E=04         0         0           0,0         -3,160474E=01         -2,48640E=04         0         0           0,18078E=06         -3,66487E=01         -2,466495E=01         0           0,18078E=06         -3,66487E=01         -3,466495E=01         0           0,18078E=05         -1,667887E=03         7,0648687E=01         -3,468496E=06         0           0,18078E=05         -1,46738E=03         7,0648687E=01         -3,468996E=06         0           0,18078E=05         -1,44678E=03         7,07048470E=01         -3,588796E=05         0           0,180776E=05         -1,44678E=03         -1,4689476E=01         -1,4689476E=05         0           0,1807776E=05         -1,468657E=03	506	ن و	.113821F=0	103582F=0	8.35692F=01	172208F	7.261077		
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10, 1983 NASTRAN	,		R2	78516	447442E-	.202273E-	587024F	.650374E-	349511E-	.832503E-	291441F	204082E-	.836370E	.353459E	.600632E-	.272153E-0	.602435E=0	4 E = 0	948184E=0	793287E-0	9E-0	657626E=0	3 1 F = 0	591122E-0	462503E-0	1.7375136-03	.628543E-0	1.648265E-0 2.357966F-0	839957	3.201979	3,500361	637446	2,355475	7 7 7	1.74951	1-098039	33977E	3.932107E-	-5.376668E-01
JANUARY 1	;	2 2	R1	2001220	1859E	1.499902	2 5	2.21	7	2.5	9 0	. 6	Ž,	7.	- 5	Ŋ	2.98	7	ŏ	7.4	วั ซั	3	1.464382E=01			2.547777E=01	2.452320E-01	.214849E-01		-7.688227E-02	1 .34/652E=04		1.677858E-01		-366496E-	-1-496834E-01	1:311296E	9-688536E-0	-4.372036E-02
IN PLATE IN COFE	**	ENVECTO	13	62465.6-	-1.340032E	0.0	0.0 -2 107001E-01	-4.452060E-01	-6.29898E-01	-7.658064E-01	-8.61575UE+01 -8.959700F-01	-8.610253E+	-7.649123E-	-6.289662E	-4-443643E	0.0	0.0	הי	-7.324448E-	-8,877284E	7.0°7.	-9.991563E-	-8.871	-5.176853E-	-2.685602E-	0.0	-2,326158E	4.49	-7.65546E	-8,615574E-	96.0	-7.651891E-	-6.310061E-	70.00	0.0		-2,601388	3.663409	4 997
10 IN BY 11 CE SPEETS, .0045	FARANETER= 3.	REAL EIG	12	40/0440	113528E	2,4182276	,9056335	3.410657	2,5188356	1.8637516	1.261958	3203528		5766956	1275174	9667996	629146	4.1498236	2.7974141		1.410278	1,461337		.084558	197657	4 • 6 7 6 8 6 <i>7</i> 1 3 - 8 2 9 6 6 5 1	.640466	3.2582921	1.864827	1.269243	2704075	902100	419890	591562	58844	2.078540	54726	1.345628	-1.103648E-03
DISTRIBUTION OF FIGU55 IN FA	i = 40°, GEUMETRY	480E+0	11	6.V8U231E=U	-2,267317E-03		1437386-0	2.625196E=0	3.758963E-0	4.475503E-0	5.095987E=0	5.116656E=0	-4.515177E-03	3.814461E-0	2.684223E=0	0.0		9.447169E-0	1.136994E-0	5.142957E-0	4.590629E-0 5.138133E-0	6.618695E-0	8-996728E-05	2.449165E-0	1.35888E-0	0.0	02624E-0	68033E-0	953196-0	98347E-0	11432E-0	4.358414E-03	35612E-0	22697E-0	133115		60123E-0	01158E-0	7.309834E=03
N ENERGY URTED CON	PARANETER = 5.747	н	1 Y P.E	ى و	9	U	9	<b>9</b> C	ر. ق د	יט		9 U	υ (	و	ט פ	9 <b>9</b> :	0	<b>.</b>	!	. ی	و ا		ن ق		•	ى دى	ق	<b>ن</b> و	و و	، دی	<b>.</b>	ی و	<b>9</b>		9	9	<b>.</b> .		<b>.</b>
MODAL STRAIN SIMPLY SUPPOR	** SHEAR I	YCLE	OINT ID.	7.10	712	713	801	808	804	802	806	208	809	610	611 611	813	901	902	904	905	906	906	606	911	915	1001	1002	£001	1005	1006	1001	6001	1010		1013	=======================================	1103	. <del></del>	1105

HATIN ENGRED STRIBLION OF 10 IN ST. 11 IN PLATE  HATIN ENGRED STRIBLION OF 10 IN ST. 11 IN PLATE  HATIN ENGRED STRIBLION OF 10 IN ST. 11 IN PLATE  1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	7 1 3 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5				
The Config. 105   The Config. 10   The	SUBCASE		00000		
ENREC CONFIG. 0557 IN ACC SPEETS, 0045 IN CORE  LAMETER E GONETIC FARACTER 3,5 ****  1,1725E00  1,271725E00  1,271725E00  1,27173E00  1,27	500 500 500 500 500 500 500 500 500 500	R2 75348E=0 76344E=0 76341E=0 7641E=0 0 17842E=0 0 15499E=0 0 14549E=0 0 14549E=0 0 14549E=0	9 - M - M		
FUERGY DISTRIBUTION OF 10 1N BY 11 IN PLATE  **RAFFIEE # 40., GEOMETRY PARAPETER # 3.5 ****  **RAFFIEE # 40., GEOMETRY PARAPETER # 3.5 ****  **A 134806+02	z	R1 426332E-02 684710E-02 10545E-01 415989E-01 496300E-01 625959E-03 966196E-03 196599E-03 196599E-03 196599E-03	. 783009E . 0 . 376571E . 0 . 671681E . 0 . 626539E . 0		
### CONFIG 055 IN FACE SHETS,  #### ONFIG 055 IN FACE SHETS,  #### CONFIG 055	IN TEATE IN CORE	13 4, 413556 = 01 -3, 661691E = 01 -3, 661691E = 01 -1, 44677E = 01 0, 0 0, 0			
AFER CONFIG 055 IN F AFAMETER = 40., GEOMET  5.74725E+06  5.74725E+06  7.27455E-03  6 8.20644E-03  6 7.275E-03  6 8.20644E-03  6 7.275E-03  6 8.80996E-03  6 9.80996E-03  7.35386E-03  6 9.80996E-03	E SPEETS P	7.2526046 1.1251326 1.3639376 1.9710656 2.0927626 0.0 0.0 0.0 0.0 0.0 0.0 0.0	00000		
######################################		11 8.206944E-03 7.274552E-03 5.957393E-03 4.318374E-03 2.028023E-03 0.0 1.945121E-03 7.396034E-03 7.396034E-03 1.005601E-02 1.00534E-03 1.00534E-03	8,775443E-03 7,353876E-03 5,063514E-03 -1,950129E-03		
	SUPPORTEC CONF SAPPORTEC CONF SAR PARAMETER ALUE = 5,7472 CLES = 3,8154	W	9999		

NASTRAN 12/14/81

JANUARY 10, 1983

MUDAL STRAIN ENERGY DISTRIBUTION OF 10 IN BY 11 IN PLATE SIMPLY SUPPORTEC CONFIG. .055 IN FACE SFEETS, .0045 IN CORE

9354E=03
335769E-03 -3.191920E-01 0.0 402806E-03 -5.481904E-01 0.0 921396E-03 -5.426038E-01 0.0 619580E-03 -5.426038E-01 0.0 619580E-03 -5.262313E-01 0.0 619580E-03 -5.262313E-01 0.0 921221E-03 -5.262313E-01 0.0 921221E-03 5.42603E-01 0.0 921221E-03 5.42603E-01 0.0 921221E-03 5.426063E-01 0.0 931526E-04 6.461326E-01 0.0 931520E-01 -4.526062E-01 0.0 9315205E-01 -2.534618E-01 0.0 9315205E-01 -2.735902E-01 0.0 9315205E-01 -3.495104E-01 0.0 9315205E-01 -3.495104E-01 0.0 9315205E-01 -3.495104E-01 0.0 9315205E-01 -3.495302E-01 0.0
402896E-01 -2.40150EE-01 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0
621396E-03 -5.426038E-01 0.0 0.12585E-03 -5.262313E-01 0.0 0.0 0.12585E-03 -5.262313E-01 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0
0.175355E-03 -3.262313E-01 0.0 0.175355E-03 -2.501273E-04 0.0 0.012525E-03 -2.501273E-04 0.0 0.012525E-03 -2.501273E-01 0.0 0.012525E-03 -2.501275E-01 0.0 0.012525E-03 -3.26205E-01 0.0 0.012505E-01 -3.362355E-02 0.0 0.012505E-01 -2.73692E-01 0.0 0.012505E-01 -2.73692E-01 0.0 0.012505E-01 -2.594412E-01 0.0 0.012505E-01 -2.59461E-01 0.0 0.012505E-01 -2.59461E-01 0.0 0.012505E-01 -2.735905E-01 0.0 0.012506E-01 -2.735905E-01 0.0 0.012606E-01 -2.735905E-01 0.0 0.012606E-01 -2.735905E-01 0.0 0.012606E-01 -3.69506E-01 0.0 0.012606E-01 -3.69506E-01 0.0 0.012606E-01 -3.69506E-01 0.0 0.012606E-01 -3.69506E-01 0.0 0.012606E-01 -3.495104E-01 0.0 0.012606SE-01 -3.496106SE-01 0.0 0.012606SE-01 -3.496106SE-
175355E=03 =2.501273E=04 6.12689E=03 3.257034E=01 873508E=04 6.463025E=01 873508E=04 6.463025E=01 873508E=04 6.463025E=01 873051E=03 3.180437E=02 873051E=01 =1.5843127E=02 873051E=01 =2.73643E=01 873676E=01 =2.73643E=01 87474E=01 =2.73644E=01 87477E=01 =2.73644E=01 87477E=01 =2.59641E=01 87525E=01 =2.59641E=01 87525E=01 =2.59641E=01 875375E=01 =2.59641E=01 8757375E=01 =2.735964E=01 8757375E=01 =2.735964E=01 8757375E=01 =2.735964E=01 8757375E=01 =2.735964E=01 8757376E=01 =2.735964E=01 8757376E=01 =2.735964E=01 8757376E=01 =2.735964E=01 875737E=01 =2.73596E=01 875737E=01 =2.73596E=01 875737E=01 =2.73596E=01 875737E=01 =2.73596E=01 875737E=01 =2.73596E=01 875737E=01 =3.73596E=01 875737E=01 =3.73596E=01 875757E=01 =3.73596E=01 875757E=
612699E=03 3.257034E=01 0.0 475221E=03 5.472549E=01 0.0 408351E=03 5.472549E=01 0.0 461797E=03 3.180317E=01 0.0 461797E=03 -1.396335E=02 0.0 4617261E=01 -2.396335E=02 0.0 497869E=01 -4.754042E=01 0.0 497869E=01 -4.754042E=01 0.0 497869E=01 -4.754042E=01 0.0 497869E=01 -4.754042E=01 0.0 497869E=01 -4.75964E=01 0.0 497869E=01 -1.45998E=01 0.0 497869E=01 -1.45998E=01 0.0 497869E=01 -1.45998E=01 0.0 497869E=01 -1.45998E=01 0.0 497869E=01 -1.453998E=01 0.0 497869E=01 -1.45399E=01 0.0 497869E=01 -2.73594E=01 0.0 497878E=01 -2.73594E=01 0.0 497878E=01 -2.73594E=01 0.0 497878E=01 -2.73594E=01 0.0 497878E=01 -3.24458E=01 0.0 497878E=01 -3.24478E=01 0.0 497878E=01 -3.24478E=01 0.0 497878E=01 -3.4478E=01 0.0 497878E=01 -3.45881E=02 0.0 497878E=01 -3.45881E=02 0.0
921221E-03 5.420063E-01 0.0 873508E-04 6.463026E-01 0.0 461797E-03 3.180317E-01 0.0 940551E-01 -1.584127E-02 0.0 941725E-03 -1.396335E-02 0.0 941725E-01 -2.784127E-01 0.0 941725E-01 -2.784121E-01 0.0 941725E-01 -4.754042E-01 0.0 941725E-01 -2.594471E-01 0.0 941725E-01 -2.594471E-01 0.0 941725E-01 -2.594471E-01 0.0 941725E-01 -2.594471E-01 0.0 941725E-01 -2.794471E-01 0.0 941725E-01 -2.79598E-01 0.0 941725E-01 -2.79598E-01 0.0 941725E-01 -2.79598E-01 0.0 941725E-01 -2.79598E-01 0.0 941725E-01 -2.79598E-01 0.0 941725E-01 -2.79598E-01 0.0 941726E-01 -3.84588E-01 0.0 941726E-01 -3.84588E-01 0.0 941726E-01 -3.84588E-01 0.0 941726E-01 -3.84588E-01 0.0 9407119E-01 -3.85018E-01 0.0 9407119E-01 -3.495104E-01 0.0 9407119E-01 -3.85018E-02 0.0 9407119E-01 -3.495104E-01 0.0 9407119E-01 -3.85018E-02 0.0 9407119E-01 -3.85018CE-03 0.0 9407119E-01 -3.85018E-02 0.0
408351E-03 5.472549E-01 0.0 408351E-03 5.472549E-01 0.0 501675E-03 -1.396335E-02 0.0 530551E-01 -2.75442E-01 0.0 461770E-01 -2.75442E-01 0.0 47869E-01 -2.754471E-01 0.0 345205E-01 -2.524471E-01 0.0 45635E-01 -2.524471E-01 0.0 45635E-01 -2.5246E-01 0.0 455812E-01 -2.73590E-01 0.0 455812E-01 -2.73590E-01 0.0 456812E-01 -2.73590E-01 0.0 456812E-01 -2.73590E-01 0.0 456812E-01 -2.73590E-01 0.0 475714E-01 -2.73590E-01 0.0 475714E-01 -2.73590E-01 0.0 475714E-01 -2.73590E-01 0.0 475714E-01 -3.45516E-01 0.0 475714E-01 -3.45516E-01 0.0 475512E-01 -3.4580E-02 0.0 475512E-01 -3.4580E-02 0.0 475512E-01 -3.4580E-02 0.0 475512E-01 -3.4581E-02 0.0
406 551 E = 0.3
461776203 - 1396335200 461726201 - 1396335200 461726201 - 1396335200 4936596201 - 13769422001 0.0 495696201 - 137696201 0.0 49695205601 - 1376996201 0.0 49695205601 - 1376996201 0.0 49695205601 - 1376996201 0.0 4969635601 - 1376996201 0.0 496963601 - 1376996201 0.0 4969636201 - 137696201 0.0 496966201 - 137696201 0.0 4969636201 - 137696201 0.0 4969636000000000000000000000000000000000
\$501675E=01 = 1.596.355E=02 0.0 \$436551E=01 = 1.586.25E=02 0.0 \$436551E=01 = 2.736524E=01 0.0 \$4376E=02 = 5.294471E=01 0.0 \$436574E=01 = 4.556.06E=01 0.0 \$436574E=01 = 4.526.06E=01 0.0 \$436774E=01 = 4.526.06E=01 0.0 \$436774E=01 = 4.526.06E=01 0.0 \$436774E=01 = 1.725186E=01 0.0 \$45640E=01 = 1.73594E=01 0.0 \$45640E=01 = 2.73594E=01 0.0 \$45640E=01 = 2.73594E=01 0.0 \$456055E=01 = 2.73594E=01 0.0 \$456055E=01 = 2.73594E=01 0.0 \$456056E=01 = 2.73596E=01 0.0 \$460506E=01 = 2.73596E=01 0.0 \$460506E=01 = 2.73596E=01 0.0 \$46050606E=01 = 2.73596E=01 0.0 \$460506E=01 = 2.73596E=01 0.0
9430531E=01 =1,54412F=02 461786PE=01 =2,75492E=01 0.0 303879E=02 =5,294471E=01 0.0 345205E=01 =4,526062E=01 0.0 345205E=01 =2,59438E=01 0.0 345205E=01 =2,59239E=01 0.0 455812E=01 2,522136E=01 0.0 455812E=01 2,722136E=01 0.0 455812E=01 2,73594E=01 0.0 459053E=01 2,73594E=01 0.0 459053E=01 1,645372E=01 0.0 496132E=01 1,645372E=01 0.0 496132E=01 1,645372E=01 0.0 496132E=01 1,645372E=01 0.0 496132E=01 1,645372E=01 0.0 496132E=01 1,633486E=01 0.0 496132E=01 1,630486E=01 0.0 496132E=01 1,630486E=01 0.0 496132E=01 1,630486E=01 0.0 496132E=01 1,63048E=01 0.0 49613E=01 2,73048E=01 0.0 49613E=01 0.0
461720E=01 =2,736524E=01 0.0 497869E=01 =4,724042E=01 0.0 4303699E=01 =4,724042E=01 0.0 456774E=01 =4,526062E=01 0.0 497835E=01 =2,59638E=01 0.0 495812E=01 =2,59638E=01 0.0 495812E=01 =2,59290076E=01 0.0 495812E=01 =3,59296E=01 0.0 4957375E=02 =3,24618E=01 0.0 4957375E=01 =1,65372E=02 0.0 495737E=01 =1,65372E=01 0.0 496737E=01 =2,73590E=01 0.0 4967374E=01 =2,73590E=01 0.0 4957124E=01 =3,49396E=01 0.0 4957124E=01 =3,4930E=01 0.0 4957124E=01 =3,59296E=01 0.0 4957124E=01 =3,495104E=01 0.0 496712E=01 =3,495104E=01 0.0
497869E=01 =4.754042E=01 0.0 303879E=02 =5.294471E=01 0.0 345205E=01 =4.550538E=01 0.0 979835E=01 =1.722188E=04 0.0 979835E=01 =1.722188E=01 0.0 979835E=01 =2.592819E=01 0.0 979835E=01 =2.598076E=01 0.0 979835E=01 =2.73594E=01 0.0 97738E=01 =3.5299E=01 0.0 97738E=01 =3.93789E=01 0.0
303679E-02 -5.294471E-01 0.0 3745205E-01 -4.526062E-01 0.0 3745205E-01 -2.526062E-01 0.0 3745205E-01 -2.52506E-01 0.0 374967E-01 -2.52506E-01 0.0 455812E-01 -2.52306E-01 0.0 455812E-01 -2.52306E-01 0.0 455812E-01 -2.73596E-01 0.0 455053E-01 -2.73596E-01 0.0 455053E-01 -3.52406E-01 0.0 45505E-01 -3.52406E-01 0.0 455052E-01 -3.52406E-01 0.0 45505E-01 -3.52606E-01 0.0 455714E-01 -3.73596E-01 0.0 455714E-01 -3.735914E-01 0.0 455714E-01 -3.735914E-01 0.0 455714E-01 -3.735914E-01 0.0 455714E-01 -3.735914E-01 0.0
456774E=01 =4.526062E=01 0.0 347485505E=01 =2.596336E=01 0.0 3474967E=01 =2.596336E=01 0.0 455812E=01 =2.59219E=01 0.0 455812E=01 4.522136E=01 0.0 455812E=01 4.522136E=01 0.0 455812E=01 2.735948E=01 0.0 922945E=01 1.655372E=02 0.0 95723E=01 1.655372E=02 0.0 95723E=01 1.655372E=01 0.0 207138E=01 1.655372E=01 0.0 207138E=01 1.655372E=01 0.0 25124E=01 2.735948E=01 0.0 25124E=01 2.735948E=01 0.0 25124E=01 2.735948E=01 0.0 250606E=01 -6.631289E=01 0.0 250606E=01 -6.531289E=01 0.0 250606E=01 -6.5312896E=01 0.0
345205E-01 -2.596338E-01 0.0 979835E-01 -1.722188E-01 0.0 455812E-01 2.59238E-01 0.0 495812E-01 4.522136E-01 0.0 49585E-01 4.749998E-01 0.0 495053E-01 2.73564E-01 0.0 922945E-01 1.66372E-01 0.0 9291433E-01 1.65372E-01 0.0 9291433E-01 1.633485E-01 0.0 921248E-01 2.73594E-01 0.0 93808E-01 2.73598E-01 0.0 93808E-01 2.73598E-01 0.0 93808E-01 2.73598E-01 0.0 93808E-01 2.737054E-01 0.0 9380931E-01 2.737054E-01 0.0 938056E-01 1.635929E-01 0.0 938056E-01 1.635939E-01 0.0 938056E-01 1.635939E-01 0.0 938056E-01 1.635939E-01 0.0 938056E-01 1.635939E-01 0.0 938056E-01 1.635939E-01 0.0 938056E-01 1.635939E-01 0.0
24949675E-01 -1.722186E-04 455812E-01 -1.722186E-04 2984967E-01 2.52909076E-01 0.0 498496E-01 4.522136E-01 0.0 498496E-01 2.735964E-01 0.0 995723E-01 3.234818E-02 0.0 995738E-01 -1.645372E-02 0.0 29433E-01 -1.4534818E-01 0.0 253884E-02 -3.244584E-01 0.0 253884E-02 -3.24458E-01 0.0 495714E-01 -2.73299E-01 0.0 051124E-01 -1.53289E-01 0.0 257125E-01 -1.53289E-01 0.0 257126E-01 -1.53289E-01 0.0 257126E-01 -1.53289E-01 0.0 257126E-01 -2.73299E-01 0.0 257126E-01 -3.45516E-01 0.0
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			PROBLEM SET	0F. 1	1825 1496 1233	1137 1415 2130	3112	2747	= 0	ን ሶን -	3586	3109	1390	.0415							
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முட்க	ı	~ ∢	67 CF	ш Ш		5918E-0 5074E-0 5400E-0	224	+ 1	46	8E+0	1 E + 0	<b>+</b> +	2E+0	49331E+02							
PLATE		2	ENERGY	₹ 3	900	9~0	5634	9877	7057	0965	6533	5691	6513	4633							
45	i	-	TOTAL	S. 1.	- 9 N	2 0 0 2 0 0			-					1.0	•						
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NI:		ıı.		7	220	0912 1003 1004	1005	900	1010	1000	100	1108	<b>~</b> =	BTCT		1					
MUDAL STRAIN ENERGY DISTRIBUTION OF 1C SIMPLY SUPPORTE CONFIG. 055 IN FACE	GEUNETRY IP			ELEMENT	222	7-7		• • •						BOS-							
10%	EUNE		•	<u> </u>																	
1801	40., 6		= HEXA		!	•				•				# HEXA							5
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ELEMENT-TIPE = HEXA	= HEXA	PARAPETERE 3.5 AAAA		SUBCASE 1
ELEMENT-10 STATEMENT IN PROBLE 9 = 2.002277£403  ELEMENT-10 STATEMENT OF ALL ELEMENTS IN SET 99 = 0.011040£402  ELEMENT-10 STATEMENT OF ALL ELEMENTS IN SET 99 = 0.011040£402  INSTITUTE	= HEXA	EMENT STRAI	N E R G I E	
NT-ID STRAINERNERGY PERCENT OF TOTAL  10.111976E+01		TOTAL ENERGY CF Total energy CF	ELEMENTS IN PROBLEM ELEMENTS IN SET	H H
10411   1,1829966100   15655   15651	- <del>-</del>	S THE	OF 101	
15502   1,131810741   15502   15504   15505	701		3153	The same and the s
10502   7,100306£400   15546   15546   15505   15506   15505   15506	100	0 m	1000	
10500   1,176400   1,580   1,176400   1,17	105	7.10	3546	
1050   1,174/0E+01   1,550   1,000		7.5001705E+0 1.31.2624E+0		• •
10508   0.592551400   3.345   10508   10508   10508   10508   105011   1.2136.351400   1.3546   105011   1.2136.351400   1.3546   105011   1.2136.351400   1.3546   105011   1.3136.351400	501	1,317470E+	0859	
10312	1001	0.49ESS36+0	2648°	
10001		+37777777 +37890877	0.55	
10605   7 (783266+00   5345   10605   1310044-01   6581   10605   1310044-01   6581   10605   1310044-01   6581   10601   1310044-01   6581   10601   1310044-01   6581   10601   131004-10   6581   10601   131004-10   6581   10601   6581   10702   6781   10702	900	1,131,142,000	. 6650	
10.605	106	7,0983266+	3545	
1000   1,174   144   1	901	7 .C06469E+	.3499	
10000   0,50406   0,5040   0	90	345031654 3467655	4581	
10612	40	3000/100 1000/100	700	
10012	900	7.102749E+0		
10 7 0 1	106	1,331923E+	6652	: :
10702   6.293402E+00   .2151       10705   6.293402E+00       10706   1.12457E+01       10707   1.12405E+01       10712   1.14052E+01       10712   1.14052E+01       10801   1.14052E+01       10802   1.14052E+01       10803   1.14052E+01       10804   1.14052E+01       10805   1.14052E+01       10805   1.14052E+01       10806   1.14062E+01       10807   1.14062E+01       10808   1.14062E+01       10809   1.14062E+01       10809   1.14062E+01       10809   1.141466E+01       10809   1.141466+01	201	1.135461E+0	5691	
10705 10705 10706 10707 10.128026+01 10.128026+01 10.1080 10.10802 10.10802 10.10802 10.10802 10.10802 10.10802 10.10802 10.10802 10.10802 10.10802 10.10803	~	6 4 0 4 6 0 4 6 0 7 E + 0	10100	
10709   1,127457E+01   5631   10709   10712   1,127457E+01   3137   10712   1,140928E+01   3137   10712   1,140928E+01   3137   10801   8,485050E+00   4225   10801   8,485050E+00   4225   10805   8,395817E+00   4193   10805   8,395817E+00   4193   10801   8,395817E+00   4193   10801   8,395817E+00   4233   10801   8,37745E+00   12581   10901   2,766571E+00   1308   10904   2,766571E+00   1308   10904   2,766571E+00   1308   10906   1308   1308   10906   1308	<b>-</b> •	0.6 7.40 56+0	C 1 1 C .	
10   10   10   10   10   10   10   10		1 1274575	1445	
10711   b_312678E+00   .3156   .2596	٠,	6.2812185	3137	•
10712   1,140928E+01   .5998   .4225   .6601   .6601   .6508E+01   .65098   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225   .6601   .6225	. ~	6.312678E+0	3156	
10801 10802 10802 10805 10805 10806 10806 10806 10807 10808	107	1.140928E+	86.05	
10802 5.674537E+00 .2550 10805 6.576517E+00 .4193 10807 5.66578E+00 .2555 10811 5.66578E+00 .2555 10812 8.47745E+00 .2555 10901 5.178328E+00 .2566 10902 2.4673432E+00 .1875 10903 2.46771E+00 .1382 10904 2.766571E+00 .1382 10906 5.211071E+00 .1382 10906 5.211071E+00 .1383	801	8.455050E+0	. 4225	
805	=	S.,C45937E+0	. 2520	The same of the sa
800 0 2395817K+00 4193 801 0 2395817K+00 4191 802 0 2552 811		5°C7E578E+0	98820	
808	901	0+3/1946930	77.79	
812	20 I	0+147014770		
811		0+10/4×00-0.	30030	· · ·
901 5.1723261001875 902 3.753261001875 903 2.60758426001802 904 2.7655716+001302 905 3.2807165+001303 906 5.2151516+001938 907 2.7653244+001303 909 2.7653244+001303		0.5064/7E+0	C3C3 •	
902 3,751432E+00 .1875 903 2,607584E+00 .1382 904 2,6071E+00 .1382 905 3,80716E+00 .1382 906 3,80716E+00 .2603 907 5,211513E+00 .2603 908 2,765324E+00 .1938 910 2,514065E+00 .1938		0+366767 F-0		
903 2.607564E+00 .1362 904 2.766571E+00 .1362 905 3.80716E+00 .2603 906 3.80869E+00 .2603 908 2.765324E+00 .1938 909 2.765324E+00 .1306 910 2.614065E+00 .1306	001		1875	
904	60. I a mindred and the state of the state o	2060158		•
905 3.280716E+00 -2603 906 5.211513E+00 -2603 907 5.211071E+00 -2603 909 2.78934E+00 -1938 910 2.614065E+00 -1306	109	2,76457	1382	
906 -5,211513E+00 .2603 907 5,211071E+00 .2603 908 3,280889E+00 .1938 910 2,14635E+00 .1383 910 2,41405E+00 .1383	60.1	3,88071	1938	
907 5.211071E+00 .2603 908 3.E80889E+00 .1938 910 2.756324E+00 .1383 910 2.41465E+00 .1880	603 000 000 000 000 000 000 000 000 000	5,21151		
906 3, E80889E+00 1938 919 2, 14635E+00 1.1383 910 2, 141787E+00 1880	60T	.5.211071E+	£092°,	
9109	607	3,6808896+0	÷	
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MODAL STRAIN ENERGY DISTRIBUTION OF 10 SIMPLY SUPPORTEC CONFIGUSS IN FACE	ON UF 10 IN B) In face speets,	BY 11 IN PLATE S, .0045 IN CORE	JANUARY 10, 1983	NASTRAN 12/14/81 PAGE	93 ::
**** SHEAR PAHANETER # 40.0 GE	GEUMETRY PARAME	FIER 3.5 RARA		SUBCASE 1	
	. H	WENT STRAIN	ENERGIES		
ELEMENT-TYPE # HEXA SUECASE	m	* TOTAL ENERGY OF ALL Total energy of all	ELEMENTS IN PROBLEM ELEMENTS IN SET 9	# 2,422684E+03 9 = 8,194045E+02	
	ELEMENT-ID	ENER	9	* .	
	10003	0508	. 2839		
	1000	19307E+	94620		
	10005	1,2732646+01	1979.		
	10001	744676+	7324		
	10008		\$0.00°		
	10010	196716+0	- 2856 - 2856		
	1001	53	1187		
	10103	£205E+0	1886		
	10104	448978E+0	5995		
	10105	8.48350E+00 9.458253E+00	306M		
	10107	46569E+0	23607		
	10108	<u> </u>	. 2677		
	10110	#422799E+0	1908		
	10111	3,1727526+00	1395		
	10202	609E9E+0	_		
	10203	70892E+0	1028	1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年,1997年	
	1021	7303E+0	1283		
	10212	148E+0			
	10301	748E+0 786E+0	1021.		
	10303	JE62720E+0	<b>~</b> .		
	10310	2 - 6 35092E+00	~ -		
	10312	445E+0	1403		
	10402	07766E+			
	10404	6 J 4 7 6 5 5 5 6 + 0 0			
	10405	668360E+0	MITMO.	***	
	10406	94354511640	1985		
	10408	. \$56076E+0	8070		
	10409		6065.		
		23745E+0	1207		
	1050	2	1215		
	10505	• •	4 C C C C C C C C C C C C C C C C C C C	かんしょう かんしょう かんしゅう 大きな かんしゅう しゅうしゅう しゅうしゅう かんしゅう かんしゅう かんしゅう しゅうしゅう かんしゅう しゅうしゅう しゅうしゅう かんしゅう しゅうしゅう かんしゅう かんしゅう かんしゅう かんしゅう しゅうしゅう しゅう	を ない は は は は は は は は は は は は は は は は は は
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1 · ··· PAGE	SUBCASE 1		m N							•				 							-		-				,	Ť.		# \$ # \$ * ;	1.5	
NAS1KAN 12/14/81	ans .		2.422684E+03 8.194045E+02			•																		•			.1	4		198		
10, 1983 N	-	S	PRUBLEM #		7267	6222	.4621 2818	1198	.1217	4616	.6218	.7270	6225	2840	1245	2669	.3409	.3863	2672	1913	.1409	1099	1086	1398	1398	1030	1027	1398	-1891	2667	3905	1010
. JANUARY 1		ENERG1E	ELEMENTS IN PF ELEMENTS IN SE	PERCENT					:			٠	:	•			• •											•				
PLATE .CORE	# #	TRAIN	ENERGY OF ALL	N-ENERGY	677	287E+	9442E+01	+	1386E+00	936+	6326E+01	379E+	6142E+01	9436+	855E+0 687E+0	E784E+00	N	373E+0	536+0	4640E+00 1873E+00	562	62956E+00	\$825E+00	74245+0	7803E+00	96551E+00	88156E+00	371E+	4405E+00 0858E+00	1803E+	9645E+0	4085E+00
7 11 IA .0045 IA	E15F= 3.5 ***	M E N T S	* TOTAL BIN TOTAL EN	BTFA	0069	1,507	11.		2,1	1.11	50.4	•	0,0	9	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4 0 6 5	14 2		v T	2	٦.	~ *	٠, -	::1	-1 :	. 5	å.		. u	4	. 3.	C 2.0
OF 10 IN B		ELE		01-10	200	10508	10509	10510	10602	10604	10605	10607	10608	10610	-10611 10702	10703	202	10707	10708	10710	10801	10803	10810	1001	10401	10903	10910	10912	11002	11004	11006	11007
	GEOWETRY IF AKAN		ν. • × × × × × × × × × × × × × × × × × × ×	ELEMENT			÷								:																	
STRAIN ENEKGY DISTRIBUTION SUPPURTED CONFIG055 IN	ER = 40.,		■IYPE = HEXA		:		:																						1		1	
ဖြင်း * မေ	PARAMETER	:	ELEMENT-TYPE SUECASE		!		:																									
NIN EN	RRR SHEAK F				1		1																					1		1	- 1	

formation	and	Anal	ysis Center	•	
PAGE 95	:				
	SCBCASE	2,422684E+03 8,194045E+02			
JANUAKY 10, 1983 - NA	7 E R G 11 E R	ELEMENIS IN PRUBLEM 8	PERCENT OF TOTAL 1892 1192 1203 1203 1203 1203 1322 7321 6248 6248 6248 7321 6649 4630 2849	33,8222	
11	# 3.5 **** EN	* TOTAL ENERGY OF ALL EL TOTAL ENERGY OF ALL EL	\$16AIN=ENERGY 4564733E+00 256848E+00 2513985E+00 1.51349E+01 1.513746E+01 1.773743E+01 1.7737745E+01 1.7737745E+01 1.773779E+01 1.773779E+01 1.773779E+01 2.511779E+01	8,194045E+02	
	APETER L E P	м	ELEMEN1-10 11011 111011 11104 111104 111107 111109	SUBTCTAL	
MUDAL STRAIN ENEMGY DISTRIBUTION OF 10 1 SIMPLY SUPPORTEC CONFIG055 IN FACE SP	AMETEK # 40., GEUMETRY :PAR E	ELEVENI-TYPE # HEXA Suecase		H HEX	
MUDAL STRAIN EI SIMPLY SUPPÜRTI	*** SHEAR PARANETER	SUE			

forma	ition	ai	nd	Anal	ysis	Cei	nte	r																				· · · · · · · · · · · · · · · · · · ·				
	NASTRAN 12/14/81 "PAGE 96	SUBCASE 1		4,374695E+03 1,464924E+03			ar i de de la compete de la co				and the same of the same and the same and the same and the same and the same of the same o	-					i d			1. 18 1. 18	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5			.,	A CONTRACTOR OF THE CONTRACTOR		•					
	JANUAKY 10, 1983 NASTRAI		NERGIES	ELEMENTS IN PROBLEM # 4.	PERCENT OF TOTAL	4524		25542	4526	**************************************	.,2498	,2515	.2124	.2124	2517	2353	i in c	2002	45.5¢	2000 2000 2000 2000	. 2079	3658	.2014	2000	5.504	3000 0000	3637	. 2235	• 2423 2428	224 2235 35	. 2138	22
	11 IN PLATE •0045 IN CORE	= 3,5 ####	ENT STRAIN E	* TOTAL ENERGY OF ALL EI TOTAL ENERGY OF ALL EI	STRAIN-ENERGY	13875 57515	1,139556+01	1,112191E+01 1,432956E+01	2	9.3820746+00	1.692959E+01	0	9.2939536+00	9.291642E+00	1065E+	1 60563746+01	6537E+	22	1 . 1851425E+01	55	9.093349E+00	15	9-27-24-6E+00 8-20-46-8E+00	5	1.5677396+01	9.160231E+00	1,5912866+01	00+32454C-6	1.054949E+01	9.772197E+00	9.152480E+00	9,751,746,400 -,558,736,401
	JUTION OF 10 IN BY 155 IN FACE SPEETS,	, GEOMETRY PARAMETER	F. L. E. Y.	HEXA 4	ELEMEN1-10	10003	50001	10008	10010	10101	10103	10104	10105	10107	1000	10111	10201	10205	10206	10208	10211	10801	10302	10306	10307	10311	10312	10401	10403	10401	10406	- 80 0
	MODAL STRAIN ENERGY DISTRIBUTION OF 10 IN SIMPLY SUPPURTEC CONFIG055 IN FACE SPEE	SHEAR PARANETER # 40.		ELEMENT-1YPE = H Succase														F1		And the state of t				-								

:					
UNITAR PARAMETER M 40., GEO	GEOMETRY PAHAMETER	HAR D. S. HARR		SUBCASE 1	
	3	MENT STRAIN	ENERGIES		
ELEMENT-TYPE = HEXA SUBCASE	<b>a</b>	* TOTAL BAERGY CF AL TOTAL BAERGY CF AL	ALL ELEMENTS IN PROBLEM ALL ELEMENTS IN SET	99 m 1,464924E+03	
	ELEMENT-1D	STRAIN-ENERGY	F.	T0TAL	
	10411	270612E+	117		
	10502	1,0957346+01	<b>⊶</b> ∧		
	10504	26917E+	2022		
	10508	1115436+	254		
· !	10510	32762E+			
	10602	547E+	E I SZ		
	10904	014E+	7077°	· · · · · · · · · · · · · · · · · · ·	
	10605	1.112934E+01 1.113006E+01	25544		
	10609	+ +	4405	A CONTRACTOR OF THE CONTRACTOR	
	10611	1,086811E+01 9,509786E+00	24.84		
	10702	9,772915E+00 1,060547E+01	.2234		٠.
	10704	9,755086E+00	.2230		
	10706	99	62133		
	10708	25E+0	1000 1000	•	
	10710	054828E+	2411		
	10712	291117E+0	4212		
	10802	9-2661536+00	2000		
	10807	66437 68879	0		
	10808	8.E31247E+00	.2019	1	
	10812	5946E0E+	5792		
	10902	8-742912E+00 8-742912E+00 1-584437E+01	1999		
	10907	8.783034E+00 9.783034E+00 9.014322E+00			
	•	AAL 1330 L. 100		アージョン コート 使いとうがく ファーカー	The second of the second